AWARD

APR 2 8 2006

PLANS

TRANSPORTATION CABINET DEPARTMENT OF HIGHWAYS

BOYD COUNTY INTERSTATE 64 (EASTBOUND)

OVER KY 180

Station 201+92.13

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BID ITEM	CLASS A	CONCRETE CLASS AA	STEEL REINFORCEMENT	STEEL STEENSORCEMENT #EPOXY COATED	PPC I-BEAM	FOUNDATION PREPARATION	います。 STRUCTURE を表現します。 STRUCTURE を表現します。 SOLID ROCK	CRUSHED AGGREGATE SLOPE PROTECTION	STARMORED EDGE	STRUCTURE GRANULAR BACKFILL	STEEL PILES HP 14x89	PILE POINTS	Line Hernit	MASONRY COATING	ELECTRICAL CONDUIT	Structure										
UNIT	C.Y.	C.Y.	LBS.	LBS.	L.F.	L.S.	C.Y.	TON	L.F.	C.Y.	L.F.	EA.	L.F.	S.Y.	L.S.											+
ntegral End Bent 1	33.3			5776				126	45	262	227	8	35	50.1 59							<u> </u>					+
ier 1	115.0		17556	120			67.95 76		-		·			157.871.45							<u> </u>					+
emi-Integral Abutment 2	39.4		,	6105			#7.55 46		45	265				49							_					+
Frank Muskien Bareer End														21.46												+
2										<u> </u>		,					2.5				-					+
																<u> </u>										+
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																<u> </u>			×.					1		+
				908771										regions regions regions research research reference reference reference							1					+
Superstructure		<u>/ 458. l</u>		107440	1504.0					<u></u>				1400	+											#
BRIDGE TOTALS	187.7	458.	17556	119441	1504.0	1	122	126	90	-527	227	8	_35	1353	(3)	3//				·						

1) see change order \$10 (1) see change order 10

(1) see change order 10

(2) This conduit moved to the W.B Bridge over Ky 180.

(3) This conduit moved to the W.B bridge by J. Deen & D.

Was added to the W.B bridge section.

It will be paid on this bridge section.

(3) Added by C.O. * 6 but paid for under the Ramp A Bridge over East Fork

(3) Added by C.O. * 6 but paid for under the Ramp A Bridge over East Fork



ITEM NUMBER 9-60.00

Title Sheet General Notes Bar Bendina Details Subsurface Data Sheet Foundation Layout Staking Layout Integral End Bent 1 Semi-Integral Abutment 2 Framing Plan PPC I-Beam, 66", Details Steel Diaphragms Superstructure Curve Offsets Construction Elevations Electrical Details SPECIAL NOTES SPECIAL PROVISIONS 69 Embankment at Bridge End Bent Structures STANDARD DRAWINGS BGX-006-08 | Stencils for Structures BGX-012-02 | Geotechnical Legend BGX-011-04 | Barrier Transition End Drainage BBP-002-04 | Bearing Details Elastomeric Bearing Pads for Prestressed Beams Neoprene Expansion Dams and Armored Edges HP 14x89 Steel Pile Guardrail Connector to Bridge End **SPECIFICATIONS** 2004 Standard Specifications for Road and Bridge Construction 2002 AASHTO Standard Specifications for Highway Bridges REVISION

INDEX OF SHEETS

BOYD

07-05

DESIGNED BY: W.R. ABBOTT

DETAILED BY: W.R. ABBOTT

I-64 EASTBOUND OVER KY 180

Commonwealth of Kentucky DEPARTMENT OF HIGHWAYS

CHECKED BY

SHEET NO.

Si

DRAWING NO.

25253

J.T. FAULKNER

J.T. FAULKNER

TITLE SHEET

OCHNER

H.W. LOCHNER, INC.

W 10 0

0

1-23 As-Builts

24-46 Design

CONSULTING ENGINEERS AND PLANNERS

GENERAL NOTES

SPECIFICATIONS:

ALL REFERENCES TO THE STANDARD SPECIFICATIONS ARE TO THE CURRENT EDITION OF THE KENTUCKY DEPARTMENT OF HIGHWAYS STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION. WITH CURRENT SUPPLEMENTAL SPECIFICATIONS.

ALL REFERENCES TO AASHTO SPECIFICATIONS ARE TO THE 2002 EDITION AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES.

DESIGN LOAD AND METHOD:

THIS BRIDGE IS DESIGNED FOR HS25 LIVE LOAD OR ALTERNATE MILITARY LOADING, WHICHEVER PRODUCES THE GREATER STRESS. THE HS25 LIVE LOAD IS ARRIVED AT BY INCREASING THE STANDARD HS20-44 TRUCK AND LANE LOADS AS SPECIFIED IN THE AASHTO SPECIFICATIONS BY 25%. ALL REINFORCED CONCRETE MEMBERS ARE DESIGNED BY THE LOAD FACTOR METHOD AS SPECIFIED IN THE CURRENT AASHTO SPECIFICATIONS.

WIND LOAD:

THIS BRIDGE IS DESIGNED FOR A WIND LOAD BASED ON A WIND VELOCITY OF 100 MPH.

MATERIALS DESIGN SPECIFICATION:

FOR CLASS "A" REINFORCED CONCRETE FOR CLASS "AA" REINFORCED" CONCRETE F'C = 3500 PSIF'C = 4000 PSI

FOR STEEL REINFORCEMENT

FY = 60000 PSI

REINFORCEMENT:

DIMENSIONS SHOWN FROM THE FACE OF CONCRETE TO BARS ARE TO CENTER OF BARS UNLESS OTHERWISE SHOWN. SPACING OF BARS IS FROM CENTER TO CENTER OF BARS. CLEAR DISTANCE TO FACE OF CONCRETE IS 2", UNLESS OTHERWISE NOTED. EPOXY COAT BARS DESIGNATED BY SUFFIX (e) IN ACCORDANCE WITH SECTION 811.10 OF THE STANDARD SPECIFICATIONS. USE STIRRUP BEND DIAMETERS FOR BARS DESIGNATED BY SUFFIX (s) IN A BILL OF REINFORCEMENT.

BEVELED EDGES:

ALL EXPOSED EDGES SHALL BE BEVELED 1/8" UNLESS OTHERWISE SHOWN.

SHOP DRAWINGS:

SUBMIT SHOP DRAWINGS THAT ARE REQUIRED BY THE PLANS AND SPECIFICATIONS DIRECTLY TO THE BRIDGE CONSULTANT. IF ANY CHANGES IN THE DESIGN PLANS ARE PROPOSED BY A FABRICATOR OR SUPPLIER, SUBMIT THOSE CHANGES TO THE BRIDGE CONSULTANT THROUGH THE CONTRACTOR. THE BRIDGE CONSULTANT SHALL PROVIDE THE DIVISION OF BRIDGE DESIGN A COPY OF THE FINAL APPROVED SHOP PLANS.

INCIDENTAL MATERIALS:

THE STRUCTURE IS TO BE COMPLETED IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS. MATERIAL OR LABOR, NOT OTHERWISE SPECIFIED, ARE TO BE CONSIDERED INCIDENTAL TO THE CONTRACT.

DIMENSIONS:

DIMENSIONS ARE FOR A NORMAL TEMPERATURE AT 60 DEGREES FAHRENHEIT. LAYOUT DIMENSIONS ARE HORIZONTAL MEASUREMENTS.

CONCRETE:

CLASS "AA" CONCRETE IS TO BE USED IN THE SUPERSTRUCTURE DECK, PARAPETS, AND DIAPHRAGMS. CLASS "A" CONCRETE IS TO BE USED IN THE SUBSTRUCTURE. PRESTRESSED GIRDER CONCRETE SHALL BE IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS.

PAYMENT FOR PRESTRESSED PRECAST CONCRETE BEAMS:

THE BASIS OF PAYMENT FOR THE PRESTRESSED CONCRETE BEAMS SHALL BE AT THE CONTRACT UNIT PRICE PER LINEAR FOOT OF BEAM, IN ACCORDANCE WITH THE SPECIFICATIONS.

FOUNDATION PRESSURE:

SPREAD FOOTING FOUNDATIONS ARE DESIGNED FOR PRESSURES AS SHOWN IN THE SPREAD FOOTING RECORD ON THE FOUNDATION LAYOUT SHEET. END BENT PILES ARE DESIGNED FOR THE MAXIMUM AXIAL LOAD AS SHOWN IN PILE RECORD ON THE FOUNDATION LAYOUT SHEET.

ELECTRICAL CONDUIT:

THE LUMP SUM BID FOR THIS ITEM SHALL INCLUDE FURNISHING ALL CONDUIT, CONCRETE MARKERS AND OTHER MATERIALS AND LABOR FOR PLACING THESE MATERIALS IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS.

PILING:

PILING SHALL BE DRIVEN TO REFUSAL. TEST PILES SHALL BE DRIVEN WHERE DESIGNATED ON THE PLANS TO DETERMINE THE LENGTH OF PILE REQUIRED. ALL TEST PILES SHALL BE ACCURATELY LOCATED SO THEY MAY BE USED IN THE FINISHED STRUCTURE.

PILE POINTS:

PILE POINTS ARE REQUIRED FOR ALL PILES. THE POINTS SHALL BE THE TYPE FOR KEYING INTO A SLOPING ROCK SURFACE. PILE POINTS SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

PILE HAMMER:

ANY COMMONLY UTILIZED HAMMER ALLOWED BY THE DIVISION OF CONSTRUCTION WILL BE ADEQUATE TO DRIVE THE PILES TO BEDROCK WITHOUT ENCOUNTERING EXCESSIVE BLOW COUNTS AND OVER-STRESSING THE PILES. THE CONTRACTOR SHALL SUBMIT HIS PILE DRIVING SYSTEM TO THE DEPARTMENT FOR APPROVAL PRIOR TO THE INSTALLATION OF THE FIRST PILE. APPROVAL OF THE PILE DRIVING SYSTEM BY THE ENGINEER WILL BE SUBJECT TO SATISFACTORY FIELD PERFORMANCE OF THE PILE DRIVING PROCEDURES.

POURING SEQUENCE:

THE POURING SEQUENCE OF THE SLAB MAY BE CHANGED WITH THE WRITTEN APPROVAL OF THE DESIGNER.

SLOPE PROTECTION:

SLOPE PROTECTION SHALL BE CRUSHED AGGREGATE SLOPE PROTECTION IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS. GEOTEXTILE FABRIC IS TO BE CONSIDERED INCIDENTAL TO CRUSHED AGGREGATE SLOPE PROTECTION.

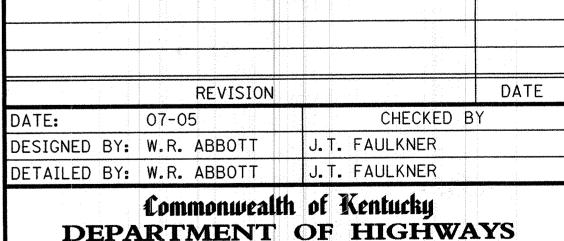
EXISTING ALUMINUM HANDRAIL:

THE EXISTING ALUMINUM HANDRAIL SHALL BE CAREFULLY REMOVED AND DELIVERED, WITHOUT DAMAGE, TO THE STATE MAINTENANCE YARD IN FRANKFORT. PAYMENT FOR THIS WORK SHALL BE INCIDENTAL TO REMOVE EXISTING STRUCTURE, LUMP SUM.

ABBREVIATIONS:

THE FOLLOWING ABBREVIATIONS MAY HAVE BEEN USED IN PREPARATION OF THESE PLANS:

bet.	BETWEEN	MPH	MILES PER HOUR
	BACK FACE	n.s.	NEAR SIDE
		O.D.	OUTSIDE DIAMETER
		0pp.	OPPOSITE
		PC	POINT OF CURVE
	CENTER TO CENTER	Perp.	PERPENDICULAR
	CURRENT EDITION	PI	POINT OF INTERSECTION
	CUBIC YARD	PPC	PRECAST PRESTRESSED CONCRETE
	CHORD	PPCDU	PRECAST PRESTRESSED CONCRETE DECK UN
	CENTER LINE	PSI	POUNDS PER SQUARE INCH
	CLEAR	PT	POINT OF TANGENT
Conc.	CONCRETE	R	RADIUS
Cu.	CUBIC	Rt.	RIGHT
	DRAWING	RCBC	REINFORCED CONCRETE BOX CULVERT
-	EACH FACE	RCDG	REINFORCED CONCRETE DECK GIRDER
	ELEVATION	Req'd.	REQUIRED
	EQUAL	RR	RAILROAD
	ESTIMATE	Shld.	SHOULDER
Ext.	EXTERIOR	spa.	SPACES
F to F	FACE TO FACE	Sta.	STATION
f.f.	FRONT FACE	Std.	STANDARD
	FAR FACE	Str.	STRAIGHT
fr.	FRONT	Tan	TANGENT
ft.	FEET	Thru	THROUGH
I.D.	INSIDE DIAMETER	TOF	TOP OF FOOTING
in.	INCH	Tot.	TOTAL
Int.	INTERIOR	Typ.	TYPICAL
Lt.	LEFT	Vert.	VERTICAL
LBS	LOW BRIDGE SEAT	W.P.	WORKING POINT
LBS.	POUNDS	Yd.	YARD
M	METER	1	
	b.f. BOF bot. Brg. C to C c.e. C.Y. Chd. Cl. Conc. Cu. Dwg. e.f. Ext. F to F f.f. f.s. ft. I.D. in. Lt. LBS LBS.	b.f. BACK FACE BOF BOTTOM OF FOOTING bot. BOTTOM Brg. BEARING C to C CENTER TO CENTER c.e. CURRENT EDITION C.Y. CUBIC YARD Chd. CHORD CL CENTER LINE CI. CLEAR CONC. CONCRETE CU. CUBIC Dwg. DRAWING e.f. EACH FACE EI. ELEVATION eq. EQUAL Est. ESTIMATE Ext. EXTERIOR F to F FACE TO FACE f.f. FRONT FACE f.s. FAR FACE fr. FRONT ft. FEET I.D. INSIDE DIAMETER in. INCH Int. LEFT LBS LOW BRIDGE SEAT LBS. POUNDS	b.f. BACK FACE BOF BOTTOM OF FOOTING Dot. BOTTOM Brg. BEARING C to C CENTER TO CENTER G.e. CURRENT EDITION C.Y. CUBIC YARD CL CENTER LINE CI. CLEAR CONC. CONCRETE CU. CUBIC Dwg. DRAWING e.f. EACH FACE EI. ELEVATION EST. ESTIMATE Ext. ESTIMATE Ext. EXTERIOR F to F FACE TO FACE f.f. FRONT FACE f.s. FAR FACE fr. FRONT ft. FEET I.D. INSIDE DIAMETER INC. D.D. D.



BOYD

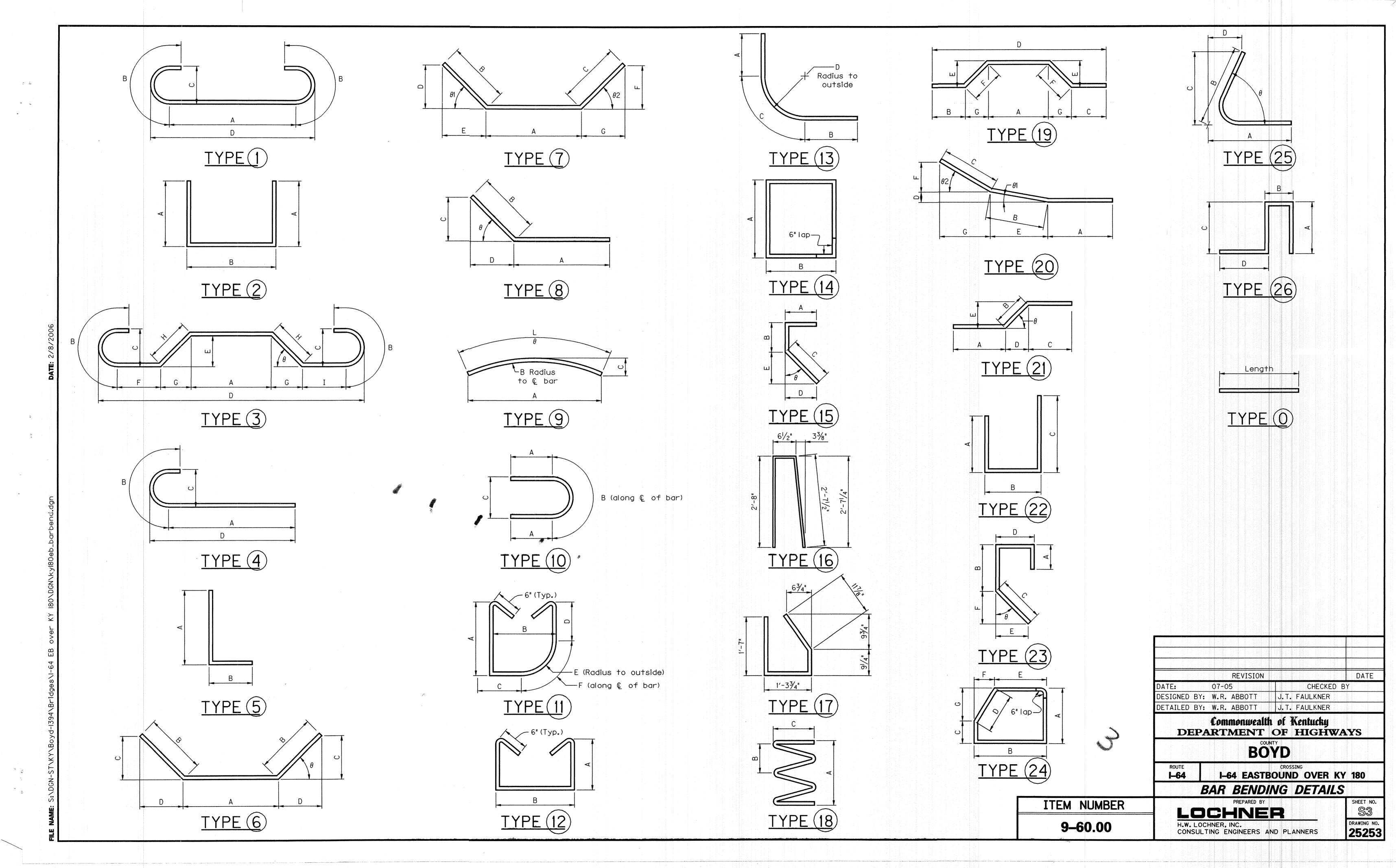
I-64 EASTBOUND OVER KY 180 GENERAL NOTES

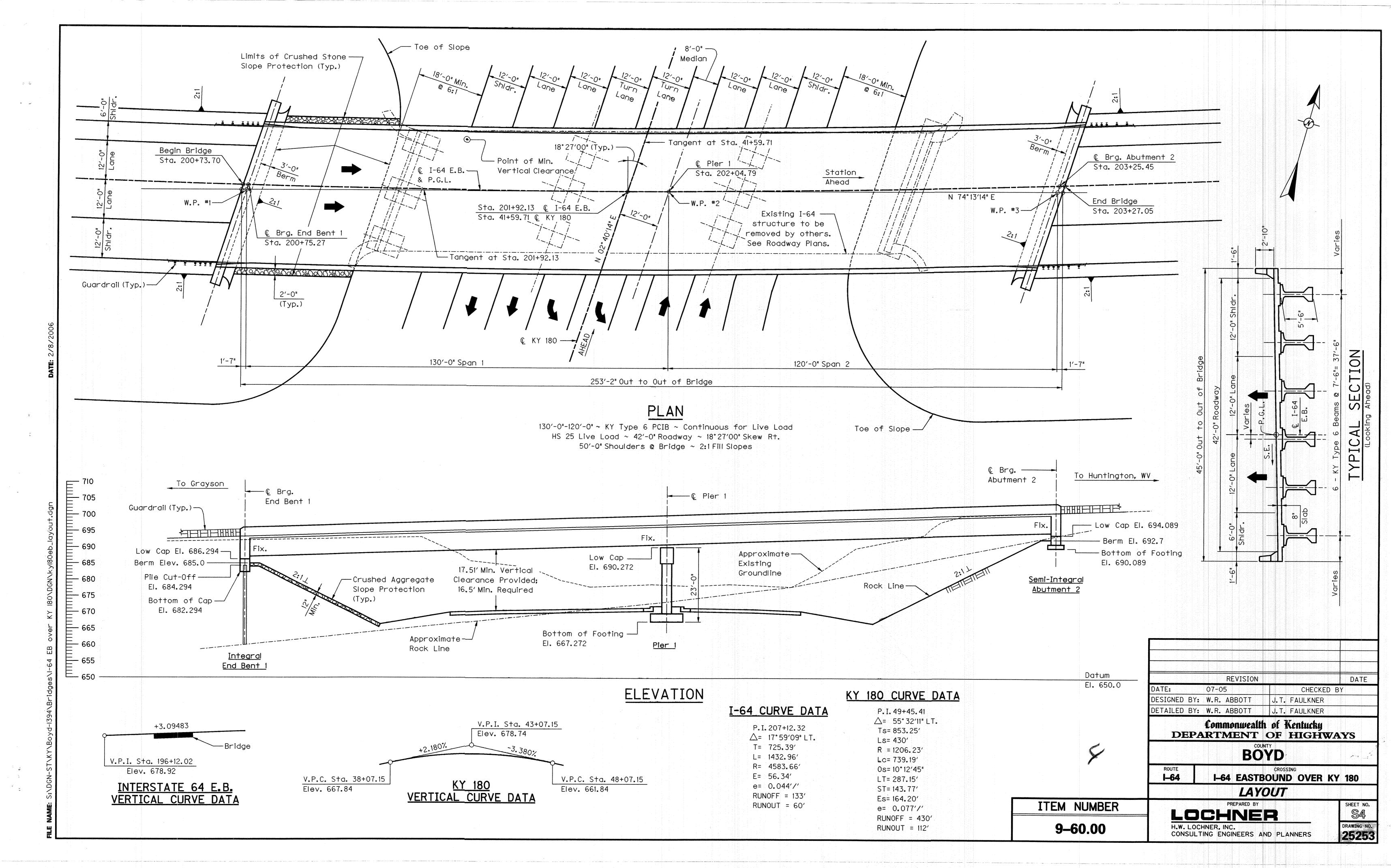
LOCHNER H.W. LOCHNER. INC. CONSULTING ENGINEERS AND PLANNERS SHEET NO.

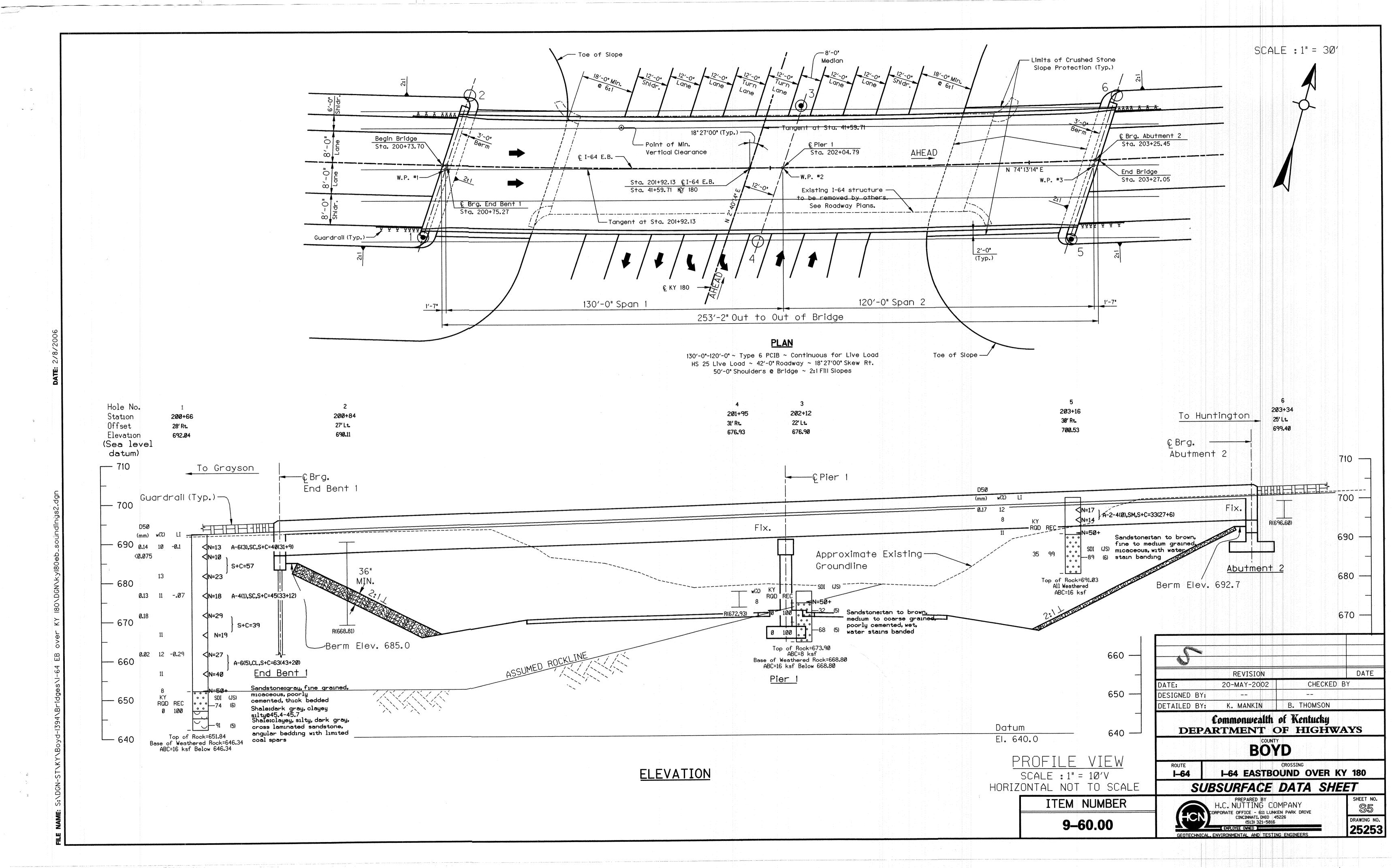
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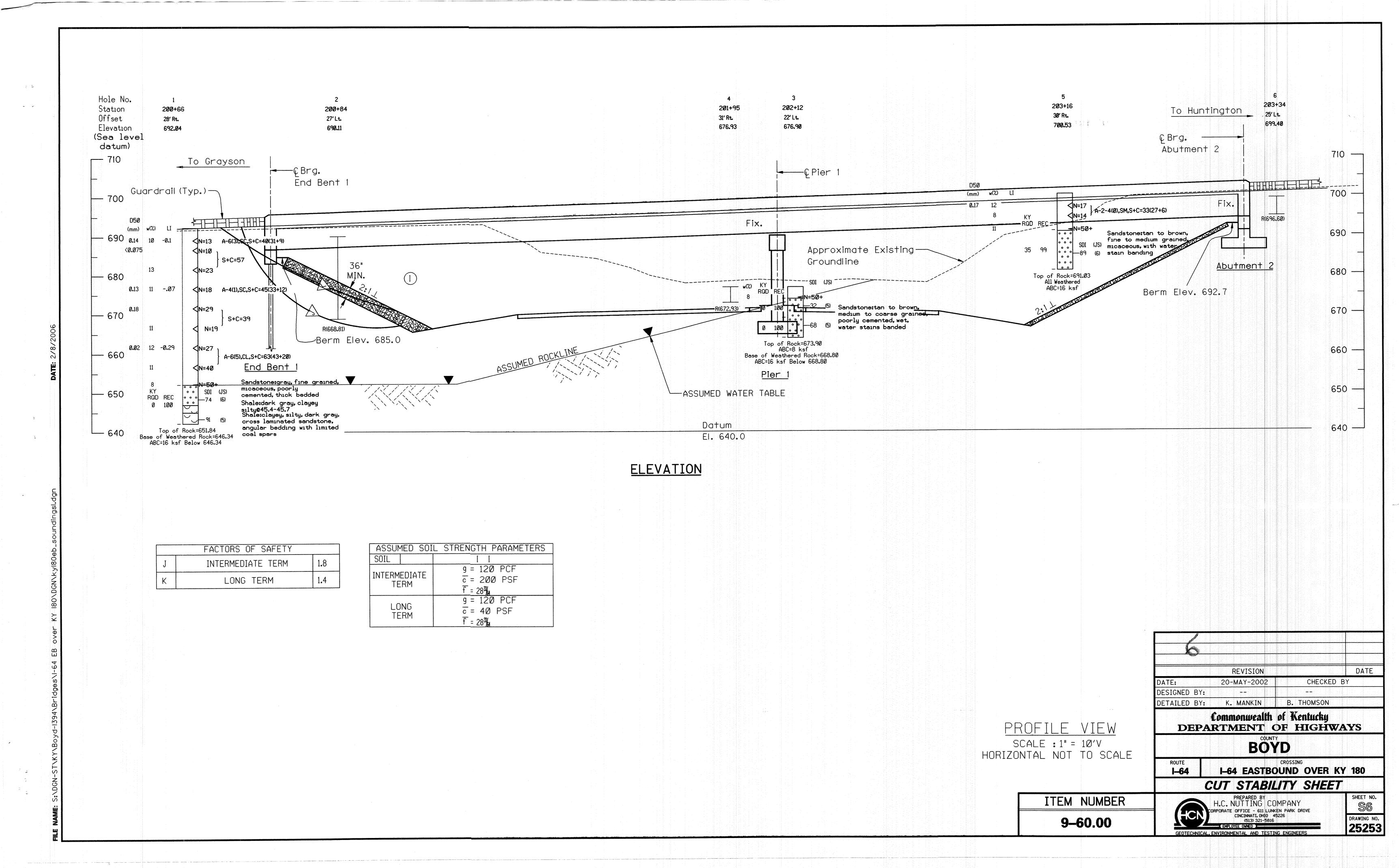
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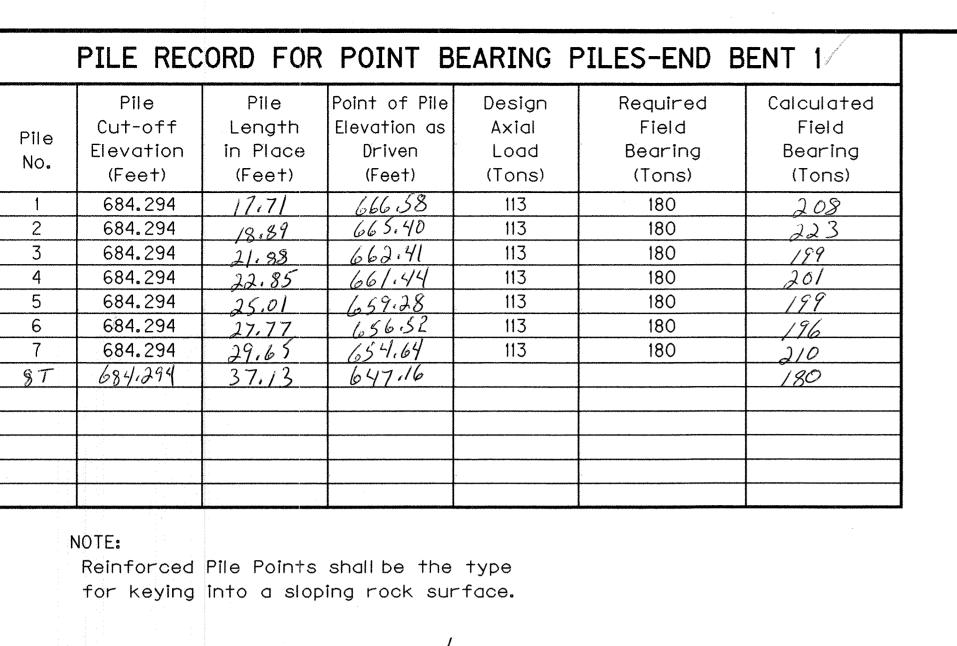
ITEM NUMBER 9-60.00











Definitions of Terms

PILE CUT-OFF ELEVATION: Elevation of the top of pile in the finished structure.

PILE LENGTH IN PLACE: Actual pile length below the Pile Cut-Off Elevation in the finished structure.

POINT OF PILE ELEVATION AS DRIVEN: Actual point of pile elvevation in the finished structure.

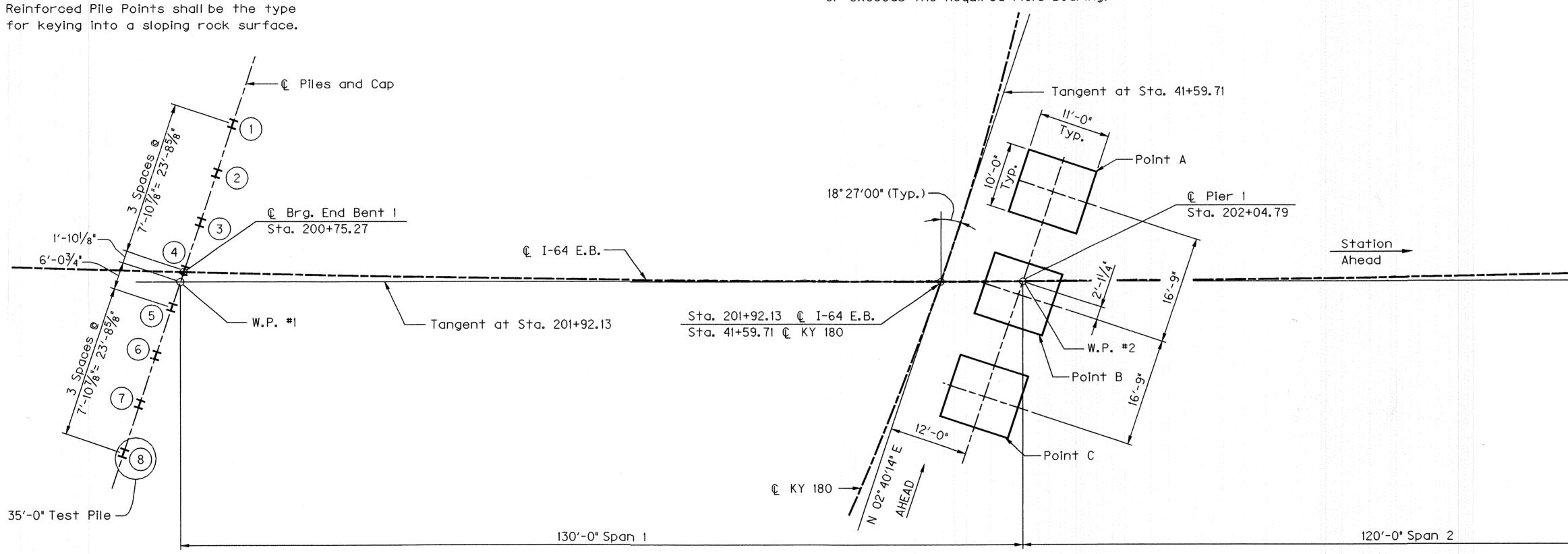
DESIGN AXIAL LOAD: Service load carried by each pile as estimated from structural design calculations.

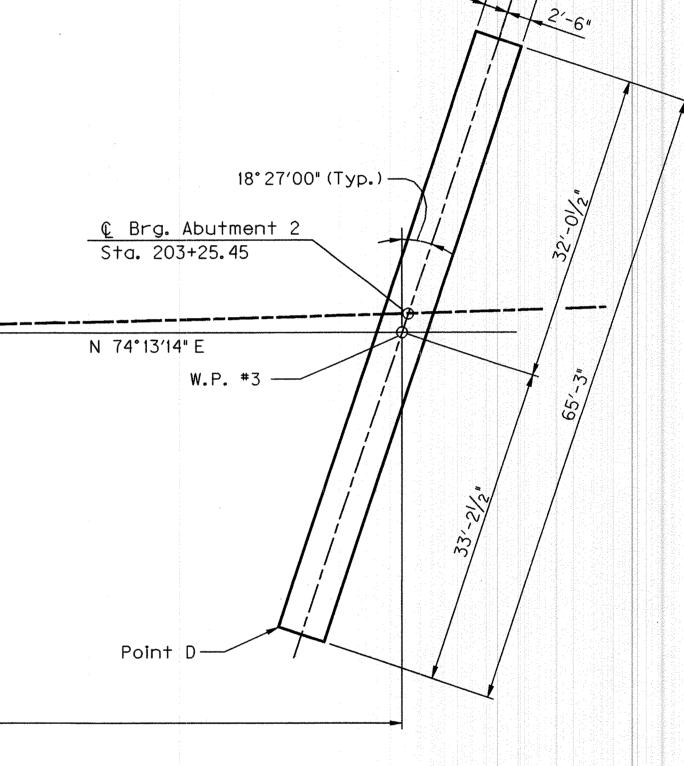
REQUIRED FIELD BEARING: Pile bearing value required to achieve "refusal" for the size of pile used. According to the Division of Construction Guidance Manual, this value is taken as 150 Tons for 12-inch steel H-Piles and 180 Tons for 14 inch steel H-Piles.

CALCULATED FIELD BEARING: Pile bearing value in place calculated using the appropriate pile driving formula in Section 604.03.07(B) of the Standard Specifications.

Driving Criteria

DRIVING CRITERIA: Drive point bearing piles to refusal and verify that the Calculated Field Bearing equals or exceeds the Required Field Bearing.





FOUNDATION PLAN

NOTE: After all foundations have been placed, the Project Resident Engineer shall record the bottom of footing elevation "As Built" and shall submit one copy of this sheet with this data to:

Kentucky Transportation Cabinet Director, Division of Bridge Design Station E3-16-01

200 Mero Street Frankfort, Ky 40622

NOTE: Contrary to the Specifications, bottom of footing elevations shall not be raised.

NOTE: If the spread footing foundation is stepped due to unsuitable material found at the given elevation, the location and elevation of the step shall be shown on this sheet and submitted along with the "As-Built" elevations.

1	EAD FOOT CORD PIE	1 /	SPREAD FOOTING RECORD ABUTMENT 2								
Point	Plan Footing Elevation	As-Built Footing Elevation	Point	Plan Footing Elevation	As-Built Footing Elevation						
Α	667.272	667,27	D	690.089	689.95						
В	667.272	667,27									
С	667.272	667.27	·								

Footing is designed for a maximum Footing is designed for a maximum pressure of 15,512 psf. Footing is designed for a maximum pressure of 9,020 psf.

is 16,000 psf.

The allowable bearing capacity

The allowable bearing capacity

is 16,000 psf.

<u>Field Data</u>

For each pile, the Project Engineer shall record the following on this sheet: Pile Length in Place, Point of Pile Elevation as Driven, and the Calculated Field Bearing. Submit this record to:

Kentucky Transportation Cabinet
Director, Division of Bridge Design
Station E3-16-01
200 Mero Street
Frankfort, Ky 40622

This pile record does not replace other pile records the Project Engineer is required to keep and submit.

Use HP 14x89 in accordance with BPS-011. c.e.

I	T	EM	NU	MBE	R
		9-	-60.	00	

	REVISION		DATE
DATE:	07-05	CHECKED BY	
DESIGNED BY:	W.R. ABBOTT	J.T. FAULKNER	
	W.R. ABBOTT	J.T. FAULKNER	

Commonwealth of Kentucky
DEPARTMENT OF HIGHWAYS
COUNTY

BOYD

CROSSING
-64 I-64 EASTBOUND OVER KY 180

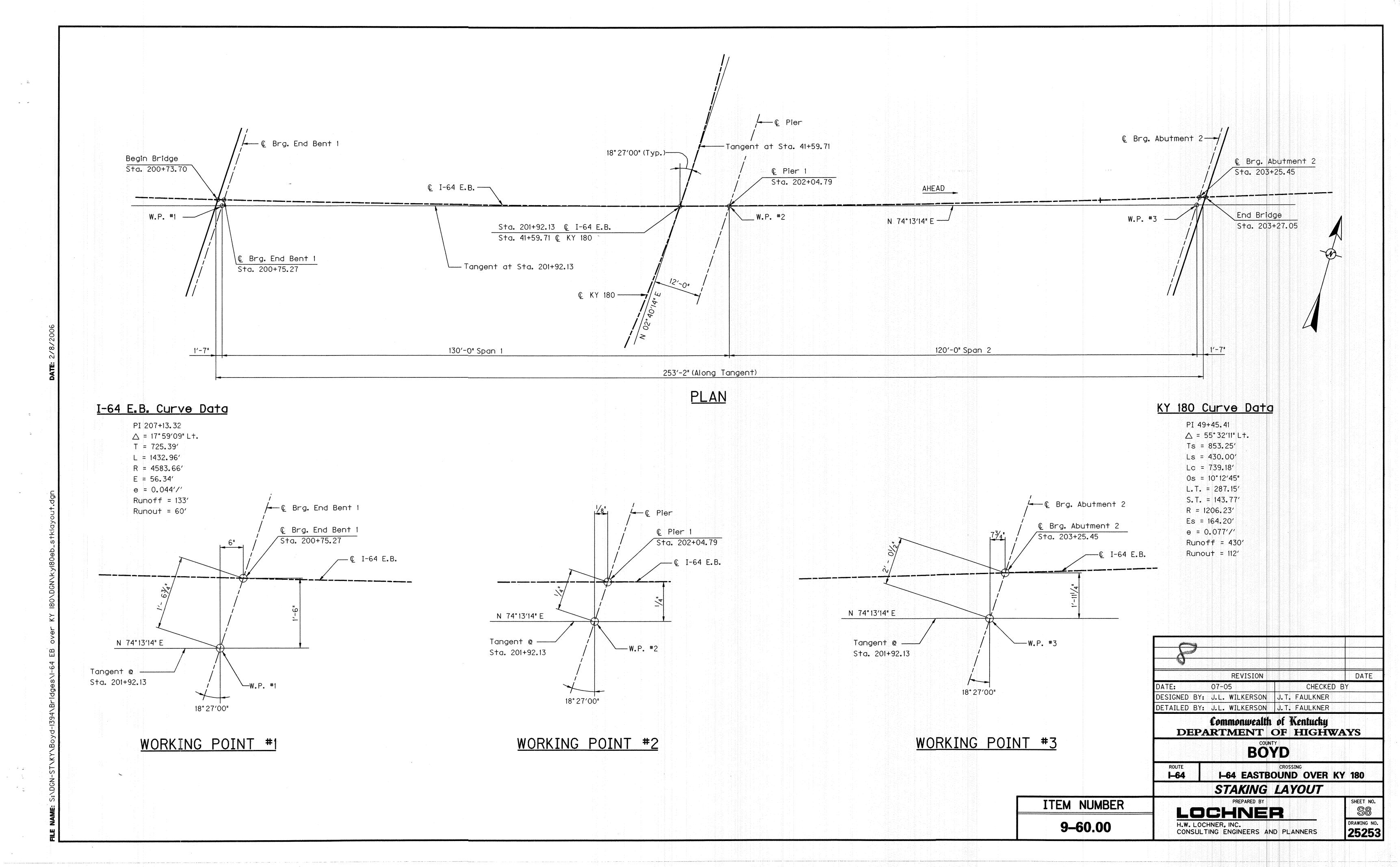
FOUNDATION LAYOUT

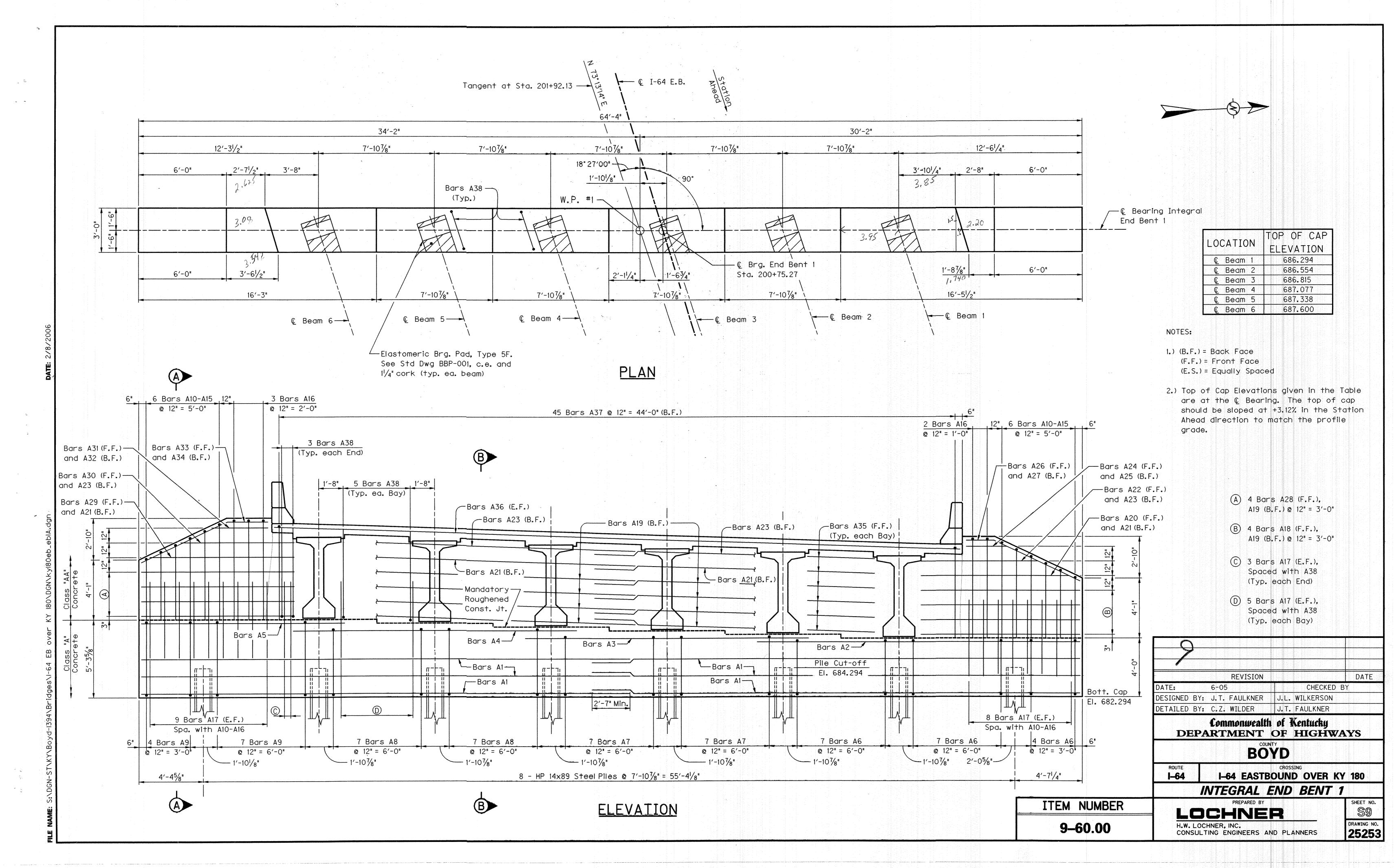
LOCHNER

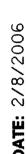
H.W. LOCHNER, INC.

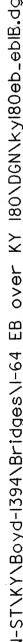
CONSULTING ENGINEERS AND PLANNERS

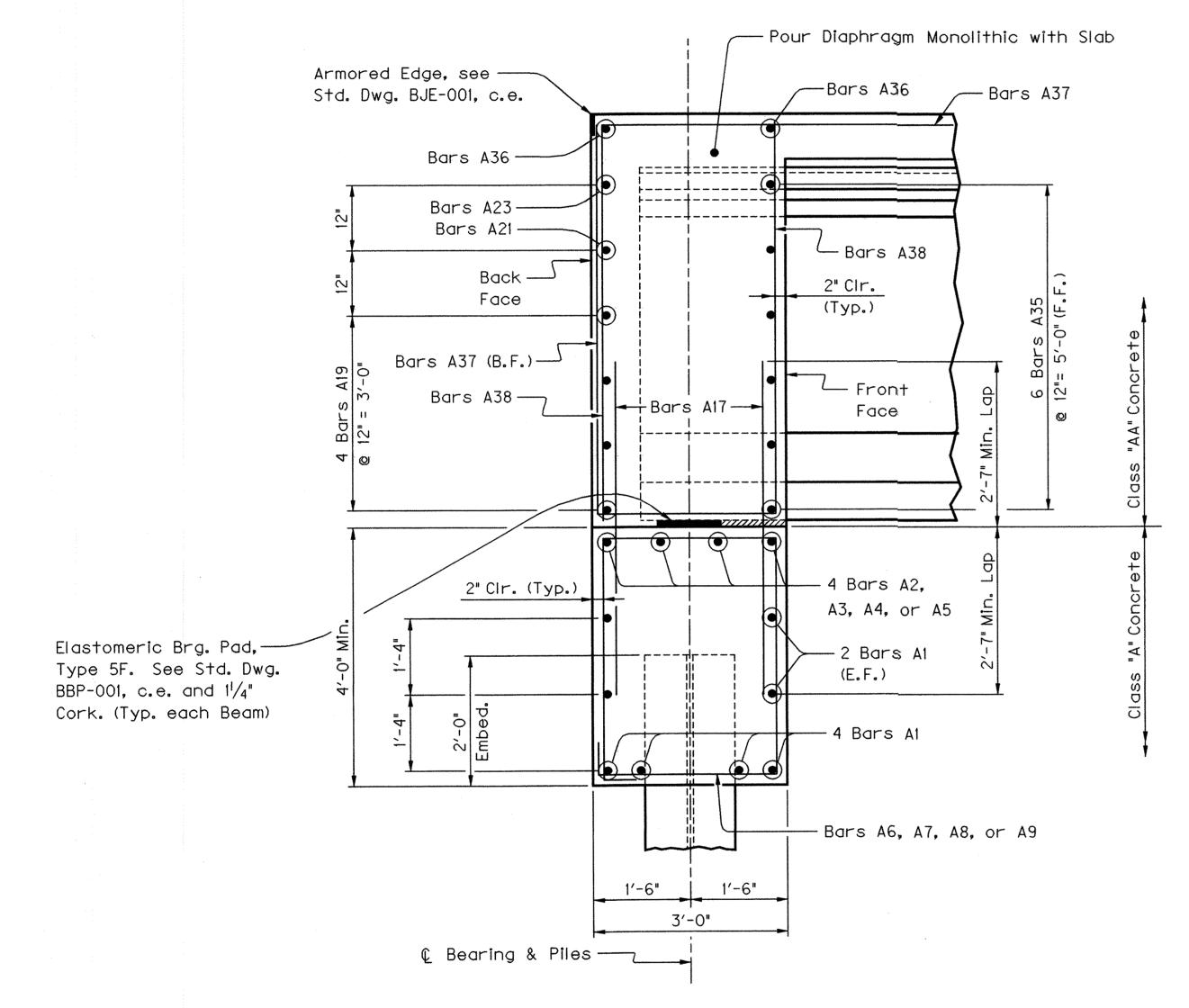
SHEET NO. S7 DRAWING NO. 25253



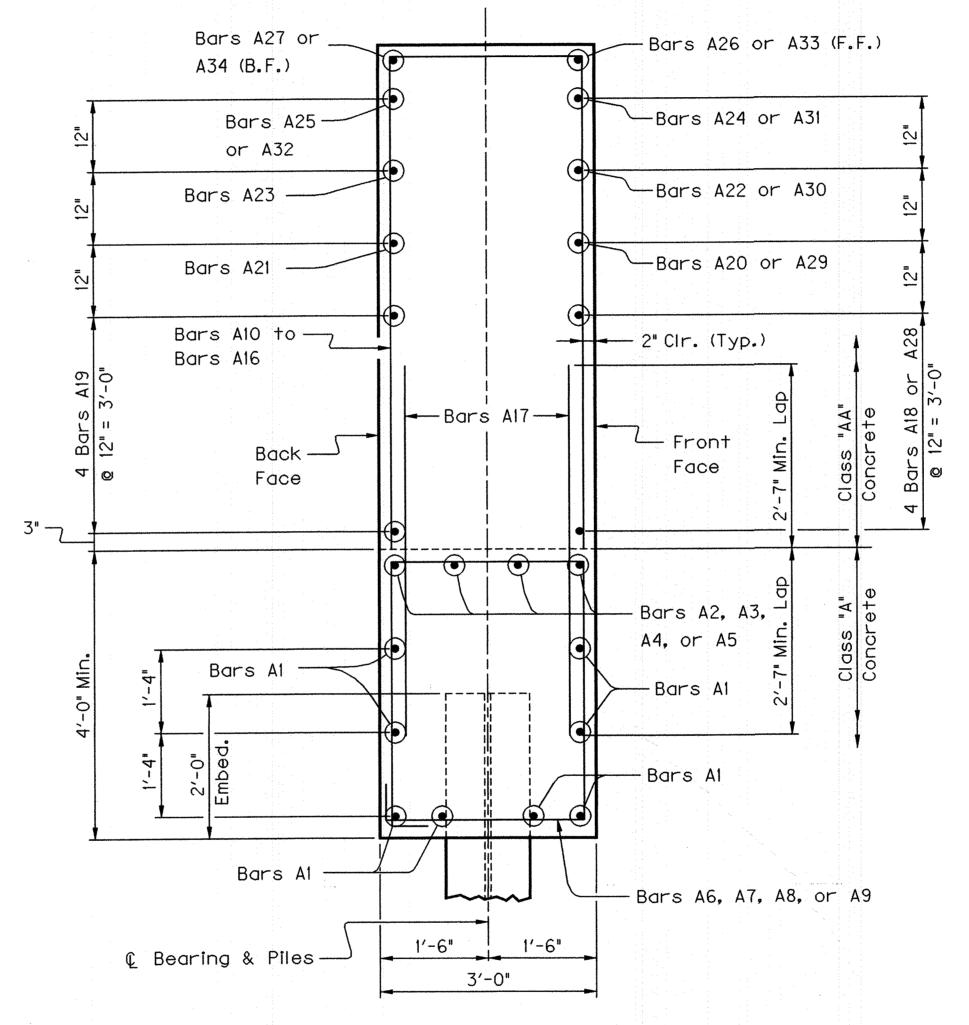








SECTION B-B



SECTION A-A

				LEN	GTH		duction of the state of the sta	Α		В		С		D
MARK	TYPE	SIZE	NO.	FT.	———	LOCATION	FT.			IN.	FT.	IN.	FT.	IN.
Ale	Str	5	16	33	4	Cap								
A2e	Str	5	4	22	6	Cap								
A3e	Str	5	4	21	0	Сар								
A4e	Str	5	4	21	0	Cap	A Company of the Comp							
A5e	Str	5	4	14	0	Cap								
A6es	14	5	18	13	3	Cap	3	7	2	8				
A7es	14	5	14	13	9	Cap	3	10	2	8				
A8es	14	5	14	14	9	Cap	4	4	2	8				
A9es	14	5	11	15	9	Сар	4	10	2	8				
A10es	2	5	2	10	8	Wing	4	01/2	2	8				
Alles	2	5	2	11	8	Wing	4	$6\frac{1}{2}$	2	8				
Al2es	2	5	2	12	7	Wing	5	0	2	8				
Al3es	2	5	2	13	6	Wing	5	51/2	2	8				
Al4es	2	5	2	14	6	Wing	5	111/2	2	8				
A15es	2	5	2	15	5	Wing	6	5	2	8				
Al6es	2	5	5	15	11	Wing	6	8	2	8				
A17e	Str	5	96	5	6	Wing/Cap								
A18e	Str	5	4	10	1	Wing								4
A19e	Str	5	8	33	4	Wing								
A20e	Str	5	1	9	5	Wing								
A21e	Str	5	2	32	9	Wing								
A22e	Str	5	1	7	3	Wing								
A23e	Str	5	2	30	7	Wing								
A24e	Str	5	1	2	9	Wing								
A25e	Str	5	1	3	5	Wing								
A26e	8	5	1	8	0	Wing	6	6	1	6	0	75/8	1	41/
A27e	8	5	1	8	11	Wing	6	6	2	5	1	03/8	2	21/2
A28e	Str	5	4	11	3	Wing								
A29e	Str		1	10	8	Wing								
A30e	Str		1	8	6	Wing								
A31e	Str	 	1	4	4	Wing								
A32e	Str	 	1	3	6	Wing								
A33e	8	5	1	9	9	Wing	6	6	3	3	ı	45/8	2	111/
A34e	8	5	1	8	11	Wing	6	6	2	5	1	03/8	2	21/
A35e	Str		30	5	3	Diaphragm								İ
A36e	Str	 	2	46	9	Slab								<u> </u>
A37e	5	8	45	16	0	Diaphragm	10	0	6	О				<u> </u>
A38es		5	31	14	9	Diaphragm	6	0	2	10				
	 	 	 	 	t						1		T I	

NOTES:

- Reinforcing Bars designated by suffix (s) shall be considered a stirup for purposes of bend diameters.
- 2.) All Class "AA" Concrete is included in Superstructure Quantities.

REVISION	DATE
DATE: 06-05	CHECKED BY
DESIGNED BY: J.T. FAULKNER	J.L. WILKERSON
DETAILED BY: C.Z. WILDER	J.T. FAULKNER

10

AILED BY: C.Z. WILDER J.T. FAULKNER

Commonwealth of Kentucky

DEPARTMENT OF HIGHWAYS

BOYD

OUTE CROSSING CROSSIN

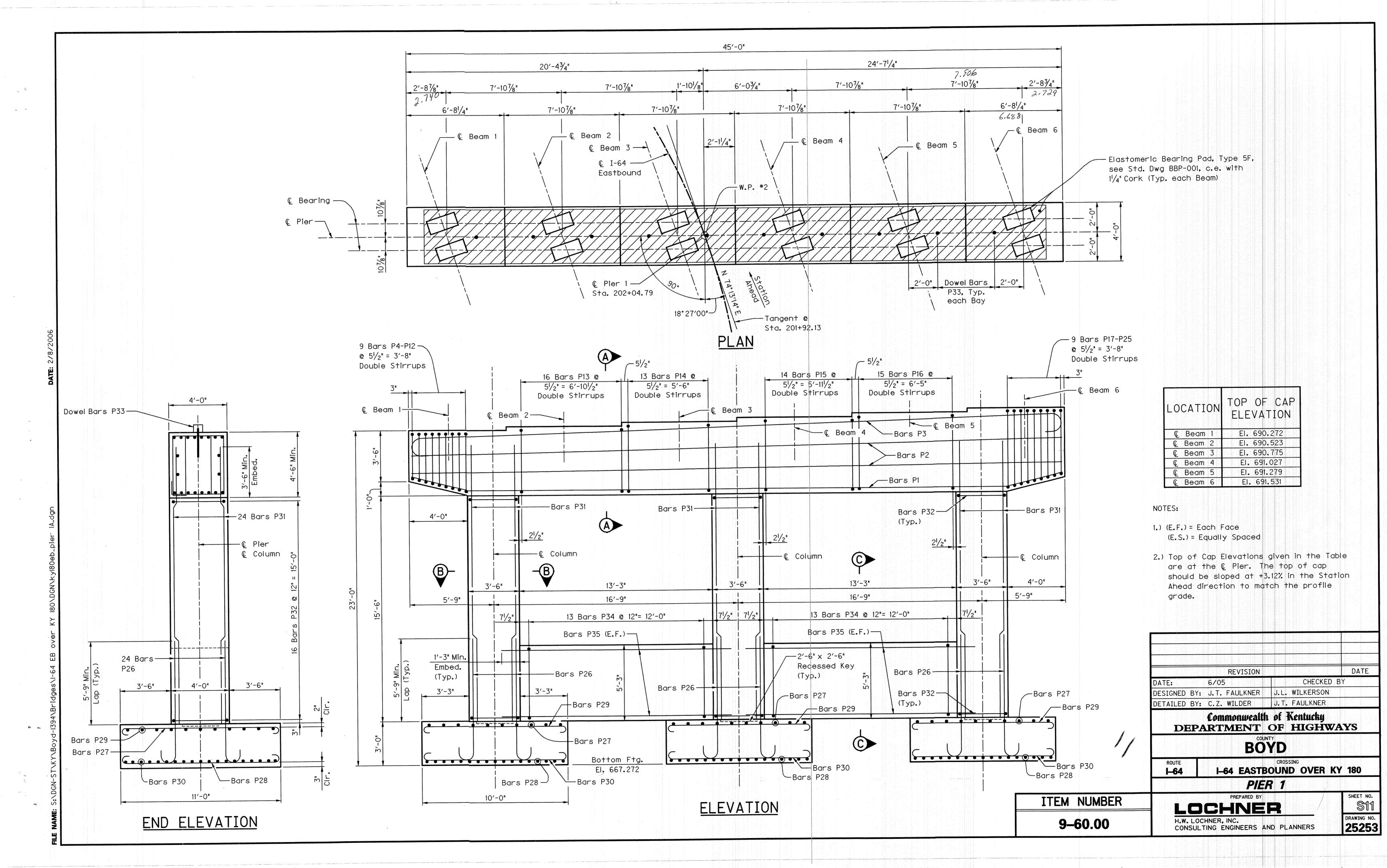
INTEGRAL END BENT 1

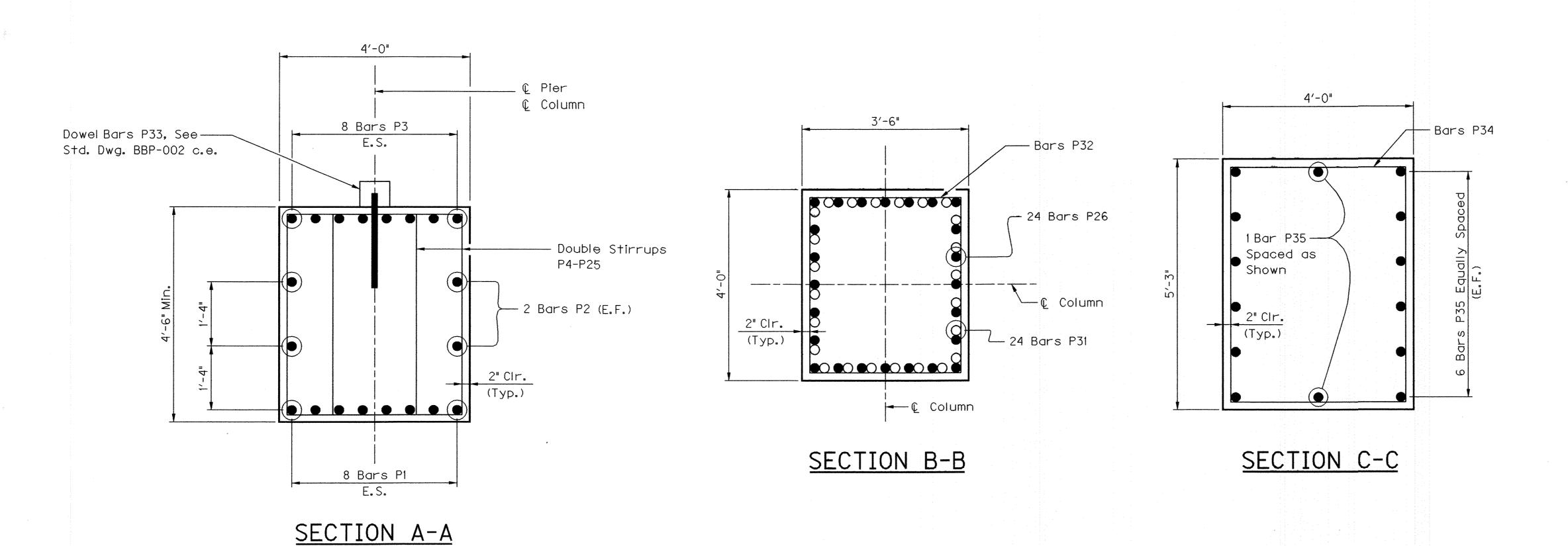
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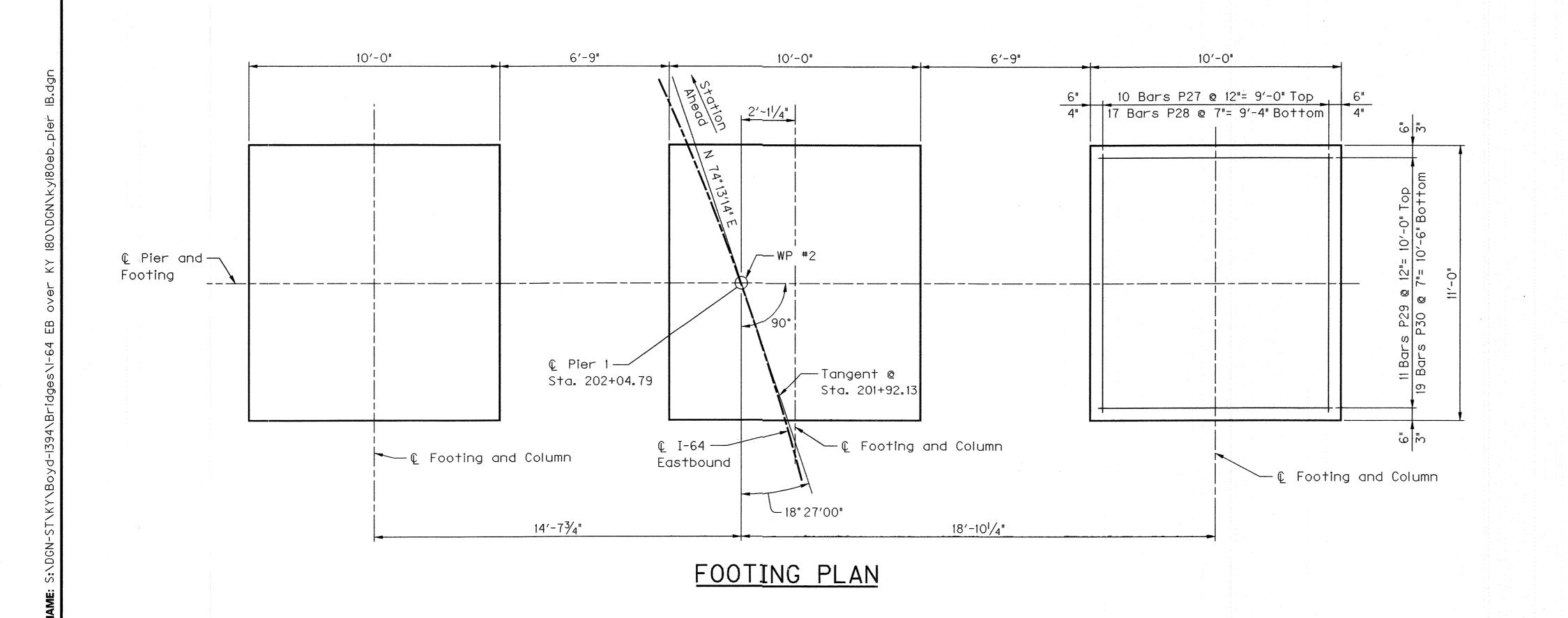
LOCHNER, INC.
CONSULTING ENGINEERS AND PLANNERS

SHEET NO.
\$10

DRAWING NO.
25253



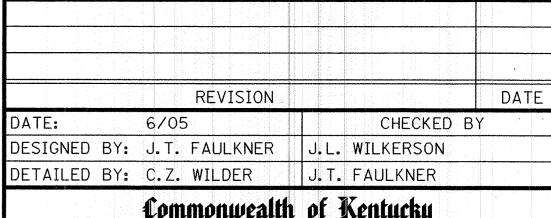




				LEN	GTH	1 0 0 1 TT 0 1 1		Α		В		C		D
MARK	TYPE	SIZE	NO.	FT.	IN.	LOCATION	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.
P1	6	9	8	44	9	Cap	36	11	3	11	0	113/8	3	95/
P2	Str	5	4	44	8	Сар								
Р3	1	11	8	48	3	Сар	43	5	2	5	1	23/4	44	73/
P4s	14	5	2	12	8	Сар	3	$2\frac{1}{2}$	2	9				
P5s	14	5	2	12	11	Cap	3	4	2	9				
P6s	14	5	2	13	2	Сар	3	$5\frac{1}{2}$	2	9				
P7s	14	5	2	13	5	Cap	3	7	2	9				
P8s	14	5	2	13	7	Cap	3	8	2	9				
P9s	14	5	2	13	10	Cap	3	91/2	2	9				
P10s	14	5	2	14	1	Сар	3	11	2	9				
Plis	14	5	2	14	4	Сар	4	01/2	2	9				
P12s	14	5	2	14	7	Cap	4	2	2	9				
P13s	14	5	32	15	1	Сар	4	5	2	9				
P14s	14	5	26	15	7	Сар	4	8	2	9				
P15s	14	5	28	16	1	Cap	4	11	2	9				
P16s	14	5	30	16	7	Сар	5	2	2	9				
P17s	14	5	2	17	1	Cap	5	5	2	9				
P18s	14	5	2	16	10	Cap	5	31/2	2	9				
P19s	14	5	2	16	7	Cap	5	2	2	9				
P20s	14	5	2	16	4	Cap	5	01/2	2	9				
P21s	14	5	-2	16	2	Сар	4	111/2	2	9				
P22s	14	5	2	15	11	Cap	4	10	2	9				
P23s	14	5	2	15	8	Сар	4	81/2	2	9				
P24s	14	5	2	15	5	Cap	4	7	2	9				
P25s	14	5	2	15	3	Сар	4	6	2	9				
P26	4	9	72	10	11	Column/Ftg.	9	0	1	11	0	113/4	9	57/
P27	1	5	30	11	11	Footing	10	3	0	10	0	5	10	8
P28	1	6	51	12	2	Footing	10	2	1	0	0	6	10	8
P29	1	5	33	10	11	Footing	9	3	0	10	0	5	9	8
P30	1	6	57	11	2	Footing	9	2	1	0	0	6	9	8
P31	Str	9	72	19	0	Column								
P32s	14	5	48	14	5	Column	3	8	3	2				
P33e	Str	*	10	2	0	Cap								
P34s	14	5	26	17	11	Crashwall	4	11	3	8				
P35	Str	5	28	15	9	Crashwall								

^{*} $1\frac{1}{2}$ "Ø Smooth Round Bar, may be commercial grade steel.

- Reinforcing bars designated by suffix (s) shall be considered a stirrup for purposes of bend diameter.
- 2) Reinforcing bars designated by suffix (e) shall be epoxy coated.



12

Commonwealth of Kentucky
DEPARTMENT OF HIGHWAYS

BOYD

PIER 1

I-64 EASTBOUND OVER KY 180

ITEM NUMBER

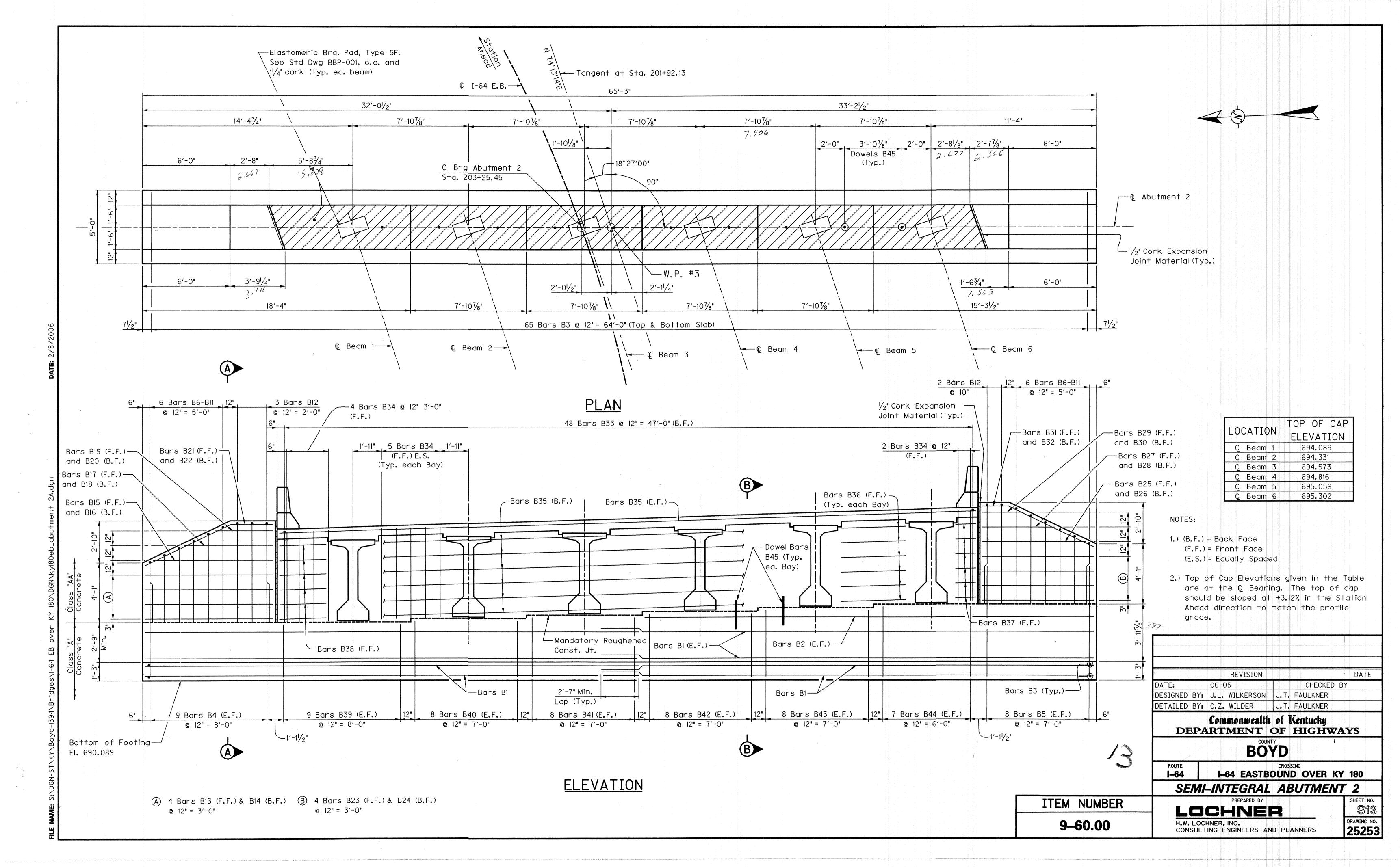
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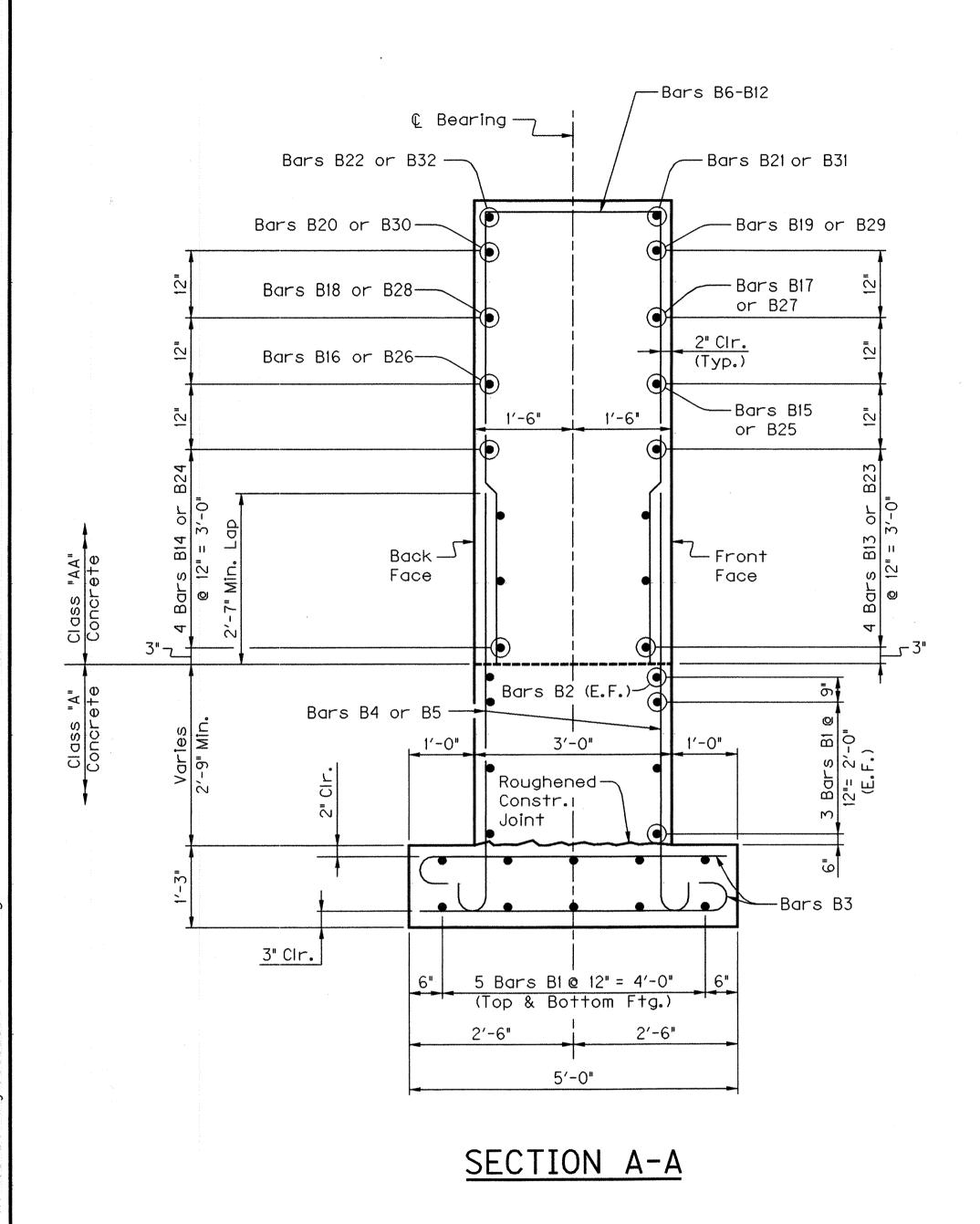
LOCHNER

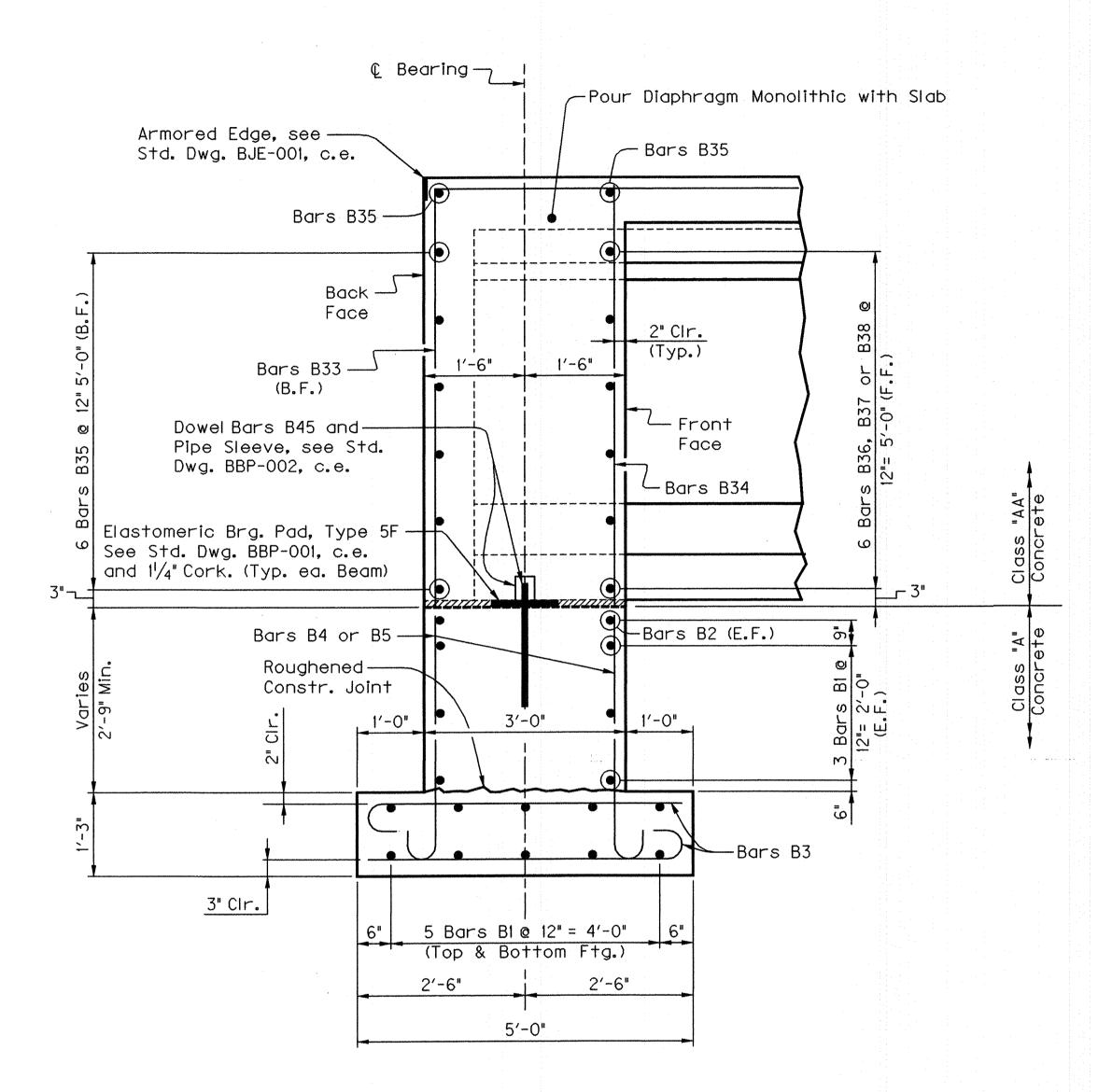
H.W. LOCHNER, INC.

CONSULTING ENGINEERS AND PLANNERS

SHEET NO. \$12 DRAWING NO. **25253**







SECTION B-B

T			

- 1.) Reinforcing Bars designated by suffix (s) shall be considered a stirup for purposes of bend diameters.
- Quantities.

/

E I A.	ILED BY: C.Z. WILDER	Ju. I. FAULKNER	
	Commonweal	lth of Kentucky	•
7	DEPARTMENT	OF HIGHWAYS	
:		OVD	•

REVISION

DESIGNED BY: J.L. WILKERSON J.T. FAULKNER

DUIU

06-05

BILL OF REINFORCEMENT

FT. IN. FT. IN. FT. IN. FT. IN.

7 | 7 | 0 | 10 | 0 | 5 | 7 | 91/2

4 | 5 | 0 | 10 | 0

6 4 0 10 0

 $4 | 0 \frac{1}{2} | 2 | 8$

 $4 | 6 \frac{1}{2} | 2 | 8$

5 0 2 8

 $5 | 5 | /_{2} | 2 | 8$

 $5 | 11 \frac{1}{2} | 2 | 8$

6 5 2 8

6 8 2 8

6 6 2 5

10 0 6 0

 $6 \ 6 \ 1 \ 4 \ 0 \ 6\frac{3}{4} \ 1 \ 2\frac{1}{2}$

6 6 2 5 1 03/8 2 21/4

3 4 0 10 0 5 3 61/2

 3
 7
 0
 10
 0
 5
 3
 9½

 3
 10
 0
 10
 0
 5
 4
 0½

 4
 1
 0
 10
 0
 5
 4
 3½

 4
 4
 0
 10
 0
 5
 4
 6½

DATE

\$14

CHECKED BY

4 7 0 10 0 5

LOCATION

Cap/Footing

Cap

Footing Cap/Footing

Cap/Footing

Wing

Diaphragm

Diaphragm

Diaphragm

Diaphragm

Diaphragm

Diaphragm

Cap/Footing

Cap/Footing

Cap/Footing

Cap/Footing

Cap/Footing

* $1\frac{1}{2}$ " Diameter Smooth Round Bar, may be Commercial Grade Steel

Cap/Footing

MARK TYPE SIZE NO. LENGTH FT. IN.

Bie Str 5 | 32 | 33 | 9

B2e Str 5 2 30 6

| B3e | 4 | 5 | 130 | 5 | 3

B4e | 4 | 5 | 18 | 7 | 2 |

| B5e | 4 | 5 | 16 | 8 | 5 |

B6es 2 5 2 10 8

B7es 2 5 2 11 8

B8es 2 5 2 12 7 B9es 2 5 2 13 6

B10es 2 5 2 14 6

Biles 2 5 2 15 5

B12es 2 5 5 15 11

B13e Str 5 4 9 4

B14e Str 5 4 8 4

B15e Str 5 1 8 8

B16e Str 5 1 7 8

B17e Str 5 1 6 6

B18e Str 5 1 5 6

B19e Str 5 1 4 5

B20e Str 5 1 3 5

B21e 8 5 1 9 11

B22e 8 5 1 8 11

B23e Str 5 4 7 2

B24e Str 5 | 4 | 8 | 3

B25e Str 5 1 6 7

B26e Str 5 | 1 | 7 | 8

B27e Str 5 1 4 5

B28e Str 5 1 5 6

B29e Str 5 | 1 | 2 | 4

B30e Str 5 1 3 5

B31e 8 5 1 7 10

B32e 8 5 1 8 11

B33e 5 8 48 16 0

B34e Str 5 31 6 0

B35e Str 5 | 8 | 47 | 7 |

B36e Str 5 | 30 | 5 | 3 |

B37e|Str| 5 | 6 | 1 | 9 |

339e | 4 | 5 | 18 | 4 | 2 |

B40e 4 5 16 4 5

B41e 4 5 16 4 8

B42e 4 5 16 4 11

B43e 4 5 16 5 2

B44e 4 5 14 5 5

B45e Str * 10 2 0 Cap/Diaphragm

DATE:

I-64 EASTBOUND OVER KY 180

SEMI-INTEGRAL ABUTMENT 2

ITEM NUMBER LOCHNER

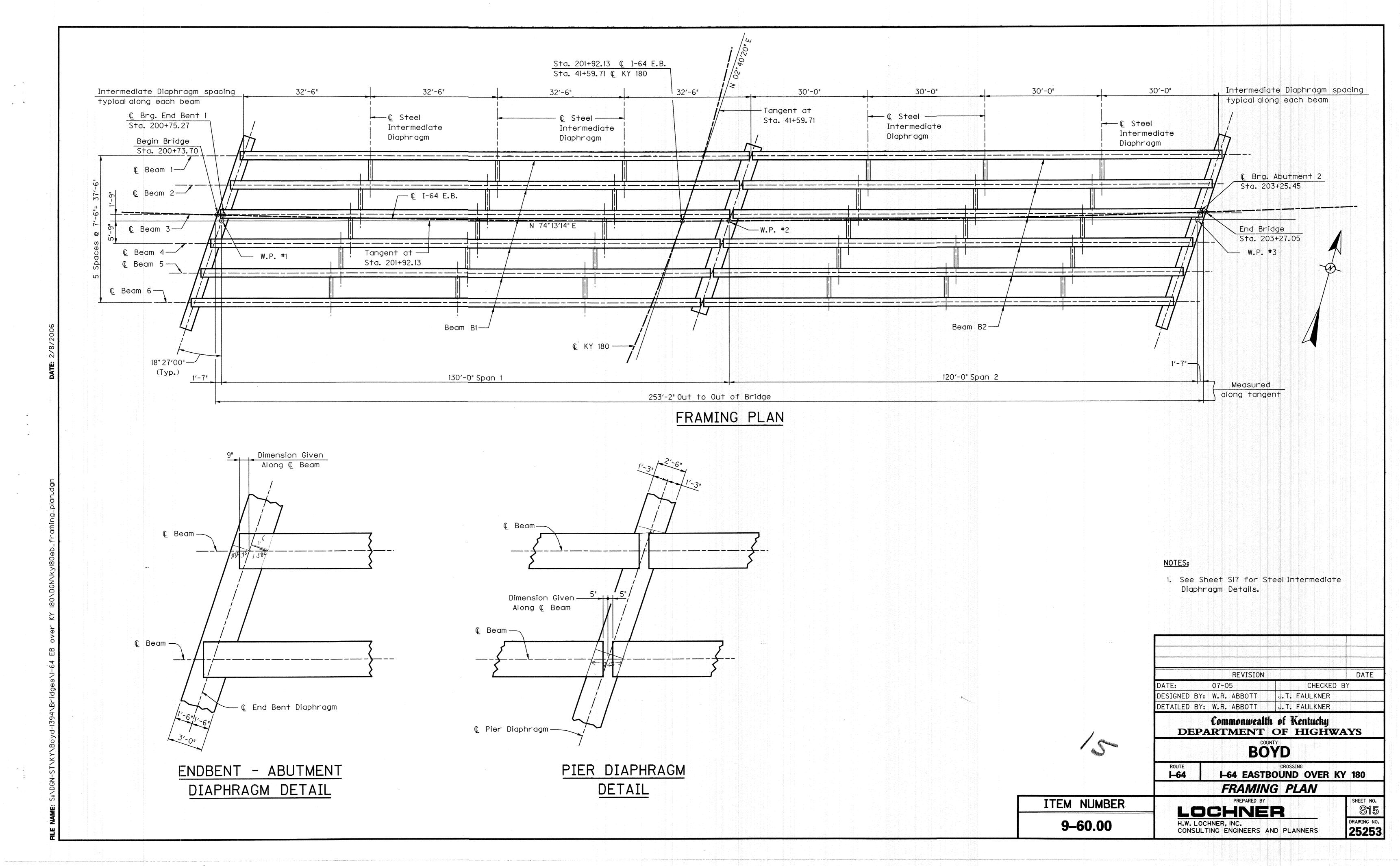
DRAWING NO. H.W. LOCHNER, INC. 25253 CONSULTING ENGINEERS AND PLANNERS

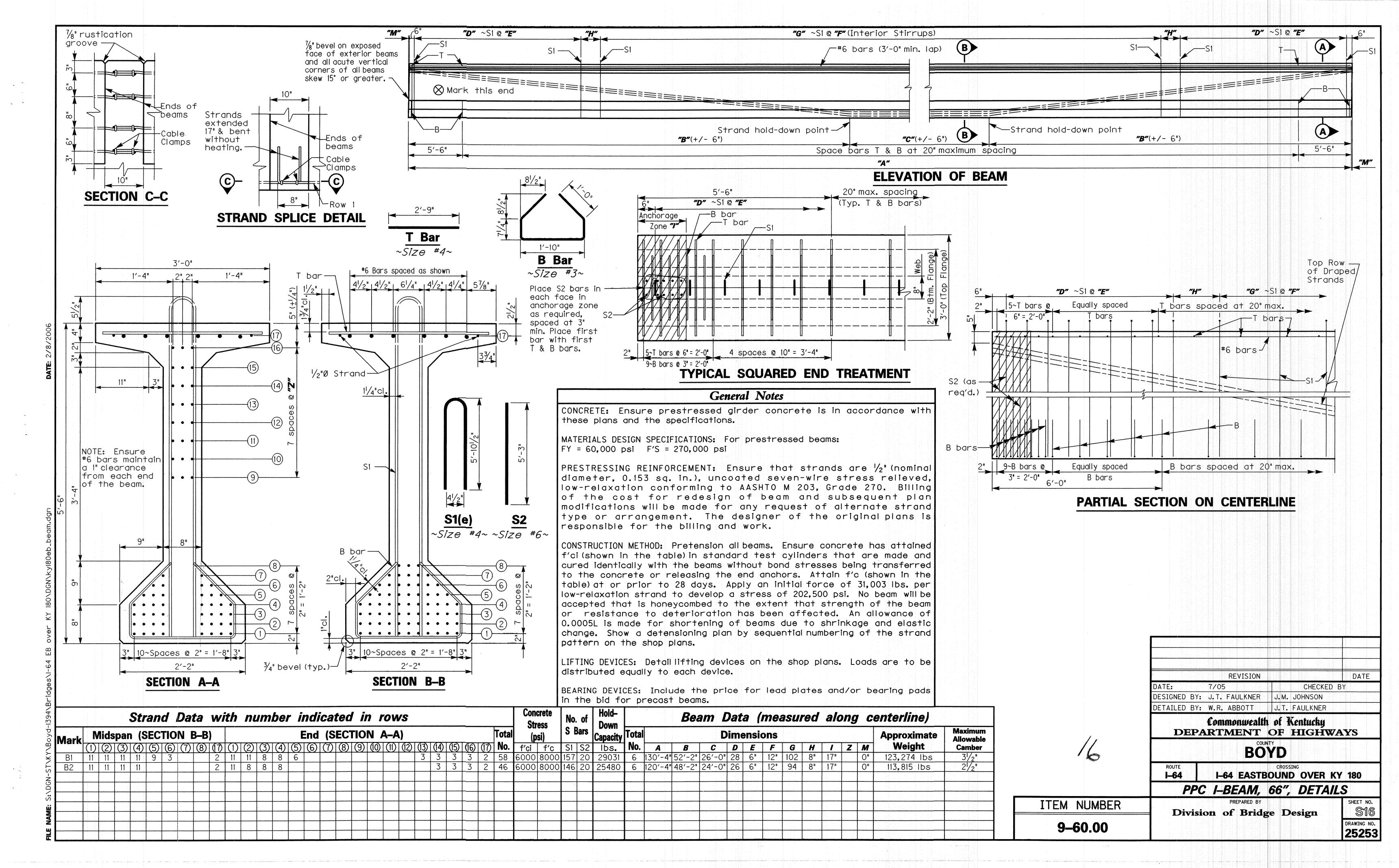
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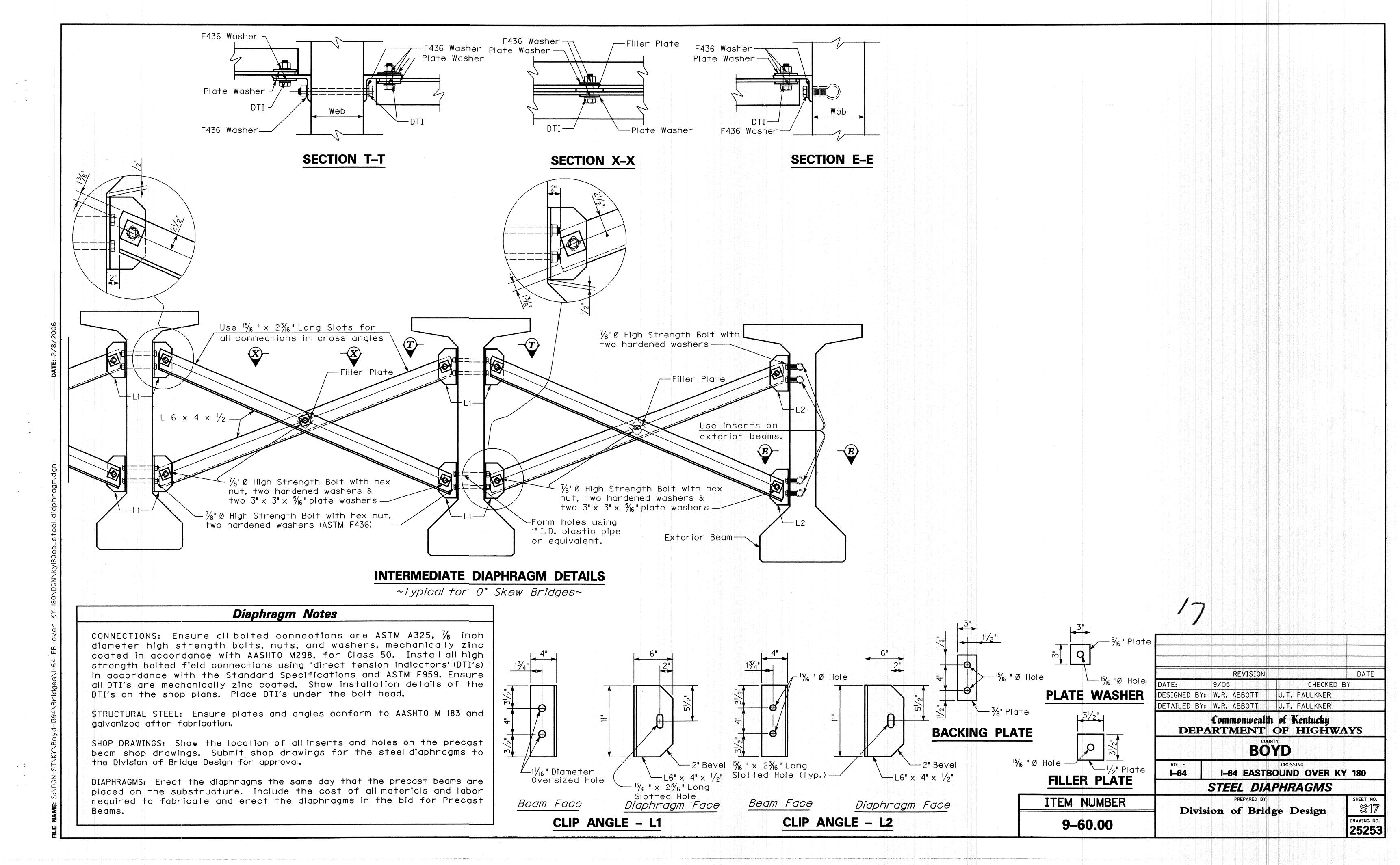
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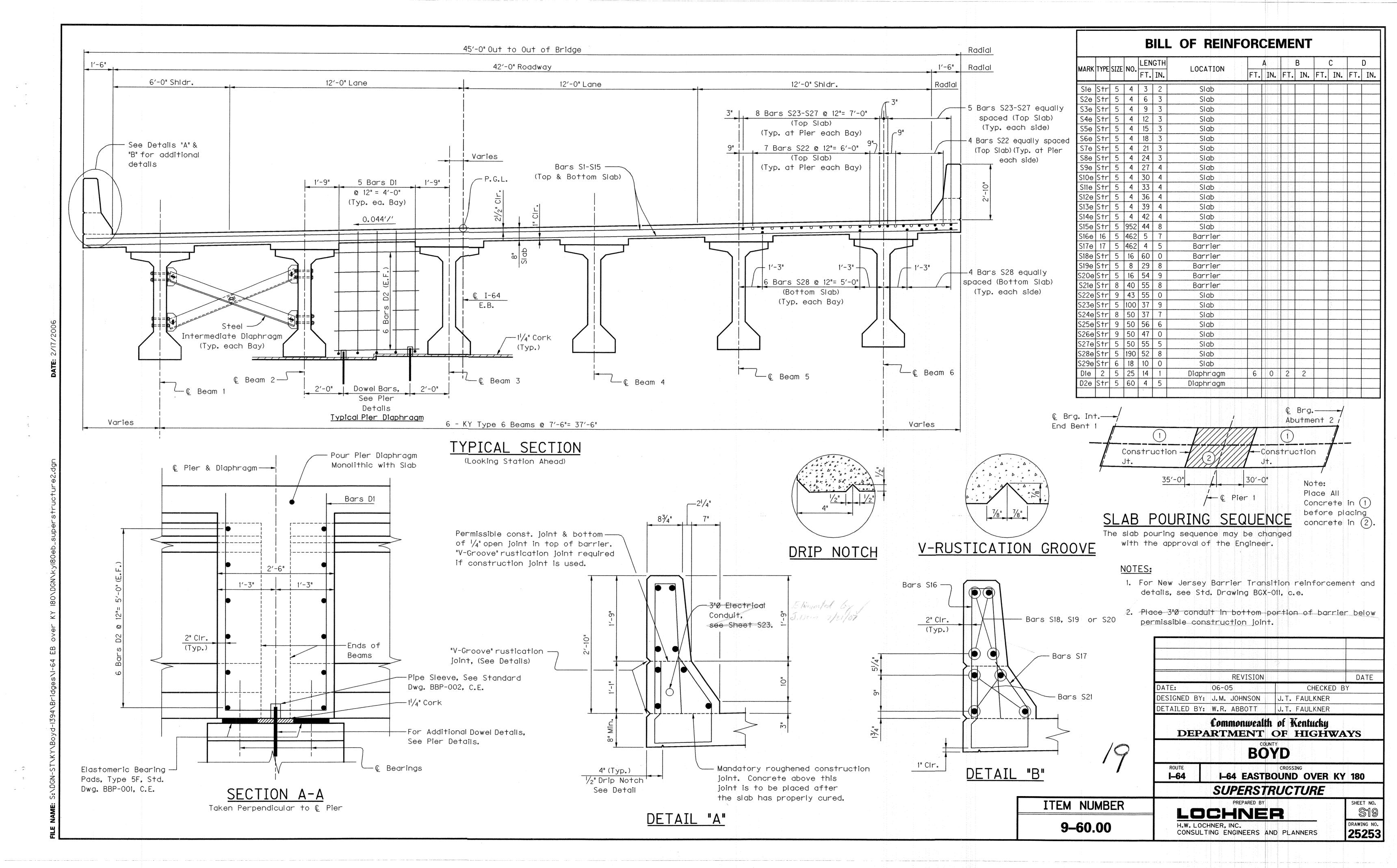
2.) All Class "AA" Concrete is included in Superstructure

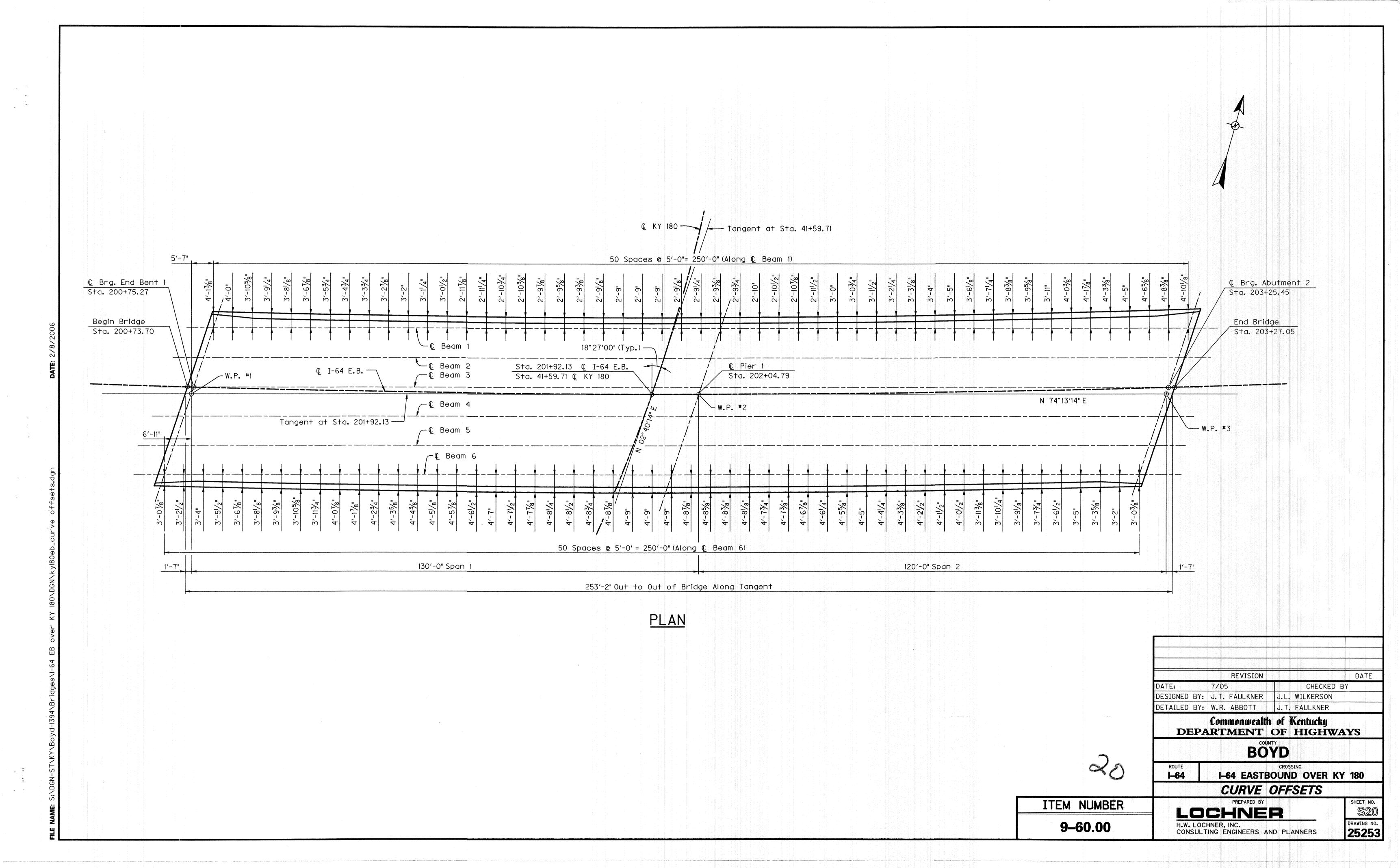
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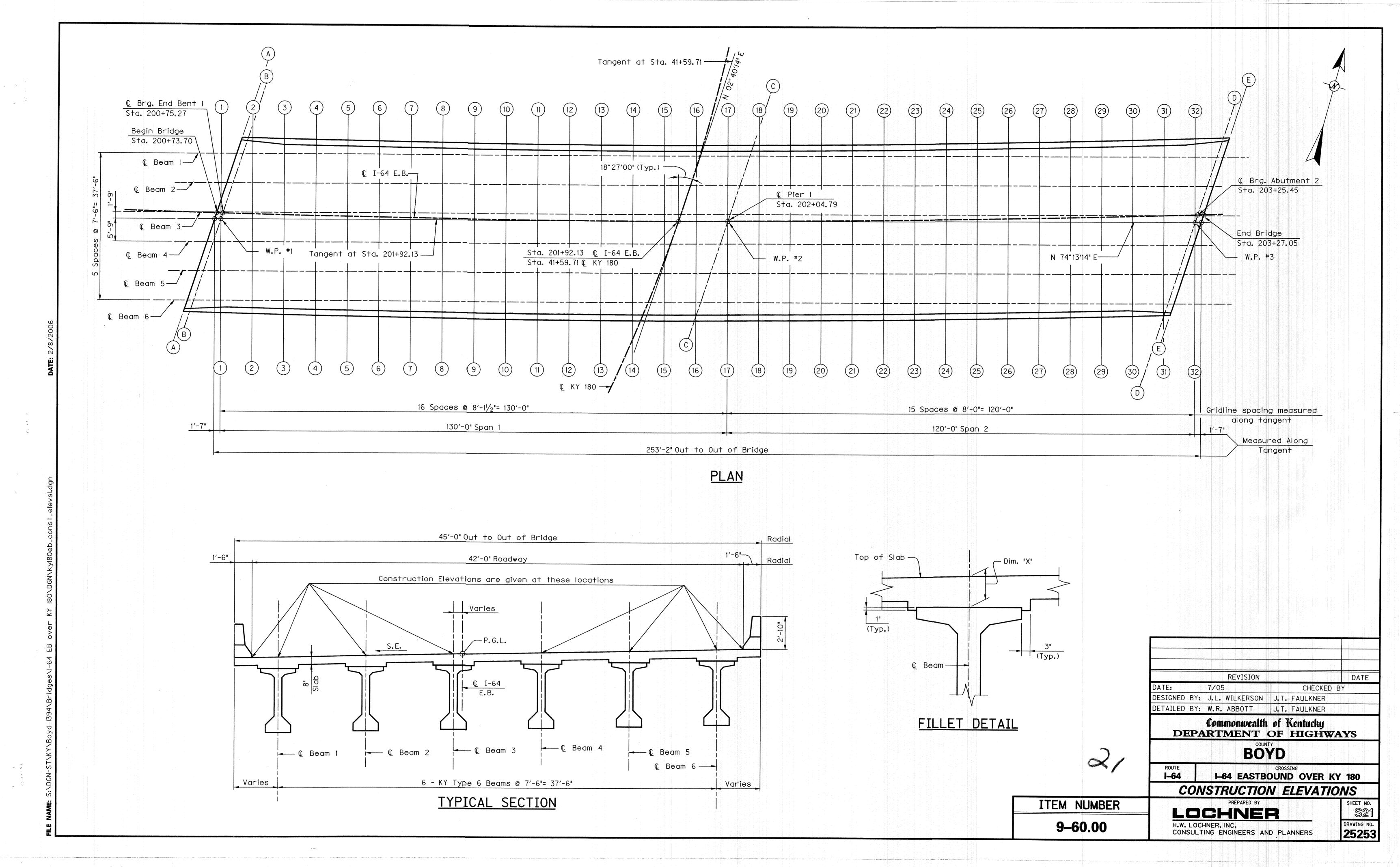












									CONSTE	RUCTIO	N ELEV	/ATIONS									
GRID	LEFT	(BEAM 1			© BEAM 2		Q	BEAM 3		(BEAM 4			E BEAM 5			© BEAM 6		RIGHT	GRID
LINE	GUTTER	CONST. ELEV.	TOP OF BEAM	DIM. "X"	CONST. ELEV.	TOP OF BEAM	DIM. "X"	CONST.	TOP OF BEAM	DIM. "X"	CONST. ELEV.	TOP OF BEAM	DIM. "X"	CONST. ELEV.	TOP OF BEAM	DIM. "X"	CONST. ELEV.	TOP OF BEAM	DIM. "X"	GUTTER	LINE
Α	692.588	 	DLAW		692.940			ELEV.	693.201	^	693.463	DLAW	. ^	693.724	DLAM	^	693.986	DLAW		694.040	A
В	692.636	 	691.93	0.80	692.987	692.16	003	692,43	693.248	0.82	693.510	692.71	,80	693.771	692.95	182	694.033	693.20	,83	694.088	В
C	696.662		695,88		696.956	1	T	696.30	697.208	0.81	697.460	696.58	188	697.712	696.86	185	697.964	697.13	183	698.073	С
D		700.522	699.21	0.81			1	700.18	701.006	0.83	701.249	700.43	182	701.492	70068	181	701.735	 	.82	701.786	D
E	700.463	 			700.815			70-10	701.057		701.300	200 10		701.543	7 52 00		701.786		· · · · · · · · · · · · · · · · · · ·	701.835	E
1						···					693.546	692.76	.79	693.925	693.11	.82	694.273	693.4B	32	694.354	. 1
2	692.696	692.816	692.00	0.82	693.163	692.38	0.78	692,72	693.501	.78	693.828	693.09	174	694.206	 	.80	694.554	†	.89	694.643	2
3	692.990	693.100	692.30	0.80	693.447	692.67	.78	693.05	693.783	.73	694.109	693.35	.76	694.486		.78	694.832	1	. 78	694.929	3
4	693.280	693.382	692.60	0.78	693.728		,76	693.34	694.063	.72	694.387	693.66	.73	694.763		.77	695.106	694.31	,80	695.211	4
5	693.568	693.662	1092.88	0.78	694.006		,76	693,60	694.339	.74	694.662	693.93	,73	695.036	 	.77	695.377	694.57	,8/	695.487	5
6	693.849	693.938		0.79	694.280		.75	693.88	694.610	.73	694.931	694.19	.74	695.302	694.53	.77	695.640		180	695.756	6
7	694.126		693.43	0.78	694.548	693.80	.75	694.12	694.876	.76	695.194	694.46	,73	695.561	694.78	.78	695.897	1095.10	,80	696.018	7
- 8	694.395	694.472		0.78	694.809	694.04	.77	654.38	695.135	.76	695.449	694.68	,77	695.815	695.03	.79	696.148	695.37	,78	696.273	8
9			693.94	0.79	695.063	694.30	.76	694,64	695.386	.75	695.697	694.94	.76	696.060	695.28	,78	696.391	695.59	,80	696.520	9
10	694.912	694.978	694.19	0.79	695.310	694.54	.77	694.90	695.630	.73	695.940	695.18	.76	696.299	695,50	.80	696.626		,81	696.760	10
11	695.160	695.221	694.45	0.77	695.550	694.79	.76	695.12	695.868	,75	696.174	695.42	,75	696.531	695.73	.80	696.856	696,06	.80	696.992	11
12	695.399	695.457	694.66	0.80	695.783	694.99	.79	695.34	696.099	.76	696.402	695.63	177	696.758	695,96	.80	697.079	696,28	180	697.217	12
13		695.688		0.77	696.012	695.20	-81		696.324	,78	696.624			696.977				696,50		697.437	13
14		695.911		0.79	696.231	695,43	.80	695.76	696.542	.78	696.841	696.09		697.192				696.70		697.652	14
15	696.078			0.79	696.450	695.63	.82	695.95	696.758		697.056				696.58		697.722	696.90	182	697.863	15
16		696.346				695.83			696.970		697.267				696.79		697.933	697.14	179	698.073	16
17	696.507					696.02			697.183	 	697.496				697.03		698.206	697.39		698.351	17
18					697.129	696.30	.83	696,63	697.458	.83	697.776				697.34			697.70		698.627	18
19	697.011	697.072	695.25	0.82	697.410	696,59	.82	696.96	697.737	1	698.054				697.62		698.759	1	177	698.900	19
20	697.289	697.352	696.57	0.78	697.688	69689	,80	697.20	698.014	,8/	698.329	697.56	177	698.697	697.90	.80		698.26	177	699.170	20
21	697.564	697.630	696.83	0.80	697.964	697.16	.80	697.50	698.289	.79	698.603	697.84	176	698.968	698.17	.80	699.300	698,54	176	699.434	21
22	697.834	697.904	697.10	0.80	698.237	697.44	.80	697.77	698.560	,79	698.871	698.10	,77		628.44				,78	699.692	
23	698.100	698.173	697.37	080	698.505	697.71	.80	698.01	698.825	.82	699.133	698.37	176	699.493	698.69			- 	.77	699.946	
24						697.97			699.084	.81	699.391	698-6P	.77		698.94		700.074	639.30	,77	700.192	24
25	698.612	698.695	691.88	0.82	699.022	698.21	.81	628.53	699.337	.81	699.641	698.88	.76		699.19				.77	700.431	25
26	698.859	698.948	698.12	0-83	699.272	898.46	281	698.78	699.585	.81	699.887	699-12	.77	700.240	659.45	.79				700.666	
27						698.72			699.828	.80	700.128				699.68			700.03		700.894	
28	699.335	699.437	698.61	0.83	699.757	698-95	181	699.26	700.065	.81	700.362	699.61	175		659.92			700.23		701.117	28
29	699.564	699.673	698,88	0.79	699.991	659.18	.81	699.50	700.297		700.593		.75		700.16			700.47	.79	701.337	29
30	699.789	699.906	659,10	0.81	700.221	699.41	.81	659.74	700.526	,79	700.820			701.166	70038		701.480	700-68		701.553	30
31	700.010		699.31	0.83	700.450	699.63	.82	699.93	700.753	.82	701.047			701.391	70057	.82	701.705	700.88	183	701.768	31
32	700.228	700.363	699.53	0.83	700.676	699.86	.82	700.16	700.979	.82											32
													Normaline di Santana di Santana anno anti Vanda						· · · · · · · · · · · · · · · · · · ·		
										:				<u></u>							

- 1.) Take elevations on top of beam at points indicated after diaphragms are in place and after forms for concrete slabs have been in place. Read elevations to three decimals using a target rod and enter reading in table under Top of Beam Elevations.
- 2.) Compute dimension "X" as follows: Construction Elevation minus Top of Beam Elevation equals Dimension "X". Construction Elevations include camber due to weight of concrete slab, barrier, and future surfacing. Measuring of Dimension "X" gives the final check on beam tolerances for camber, beam damage and errors in erection that produce reverse camber, sags, and unsightly fascia beams.
- 3.) For setting templates, measure dimension "X" above top of beam for top of template. Do not set template by elevation.
- 4.) Construct barrier to roadway grade. Do not add camber to barrier.
- 5.) Note To Resident: The "Maximum Allowable Camber" shown on the beam sheet is the amount of camber, measured prior to casting the deck, above which the beam will begin to encroach into the slab.

REVISION DATE DATE: 11/05 CHECKED BY DESIGNED BY: J.L. WILKERSON J.T. FAULKNER

Commonwealth of Kentucky DEPARTMENT OF HIGHWAYS

BOYD

DETAILED BY: W.R. ABBOTT J.T. FAULKNER

I-64 EASTBOUND OVER KY 180

SHEET NO.

S22 Drawing no.

25253

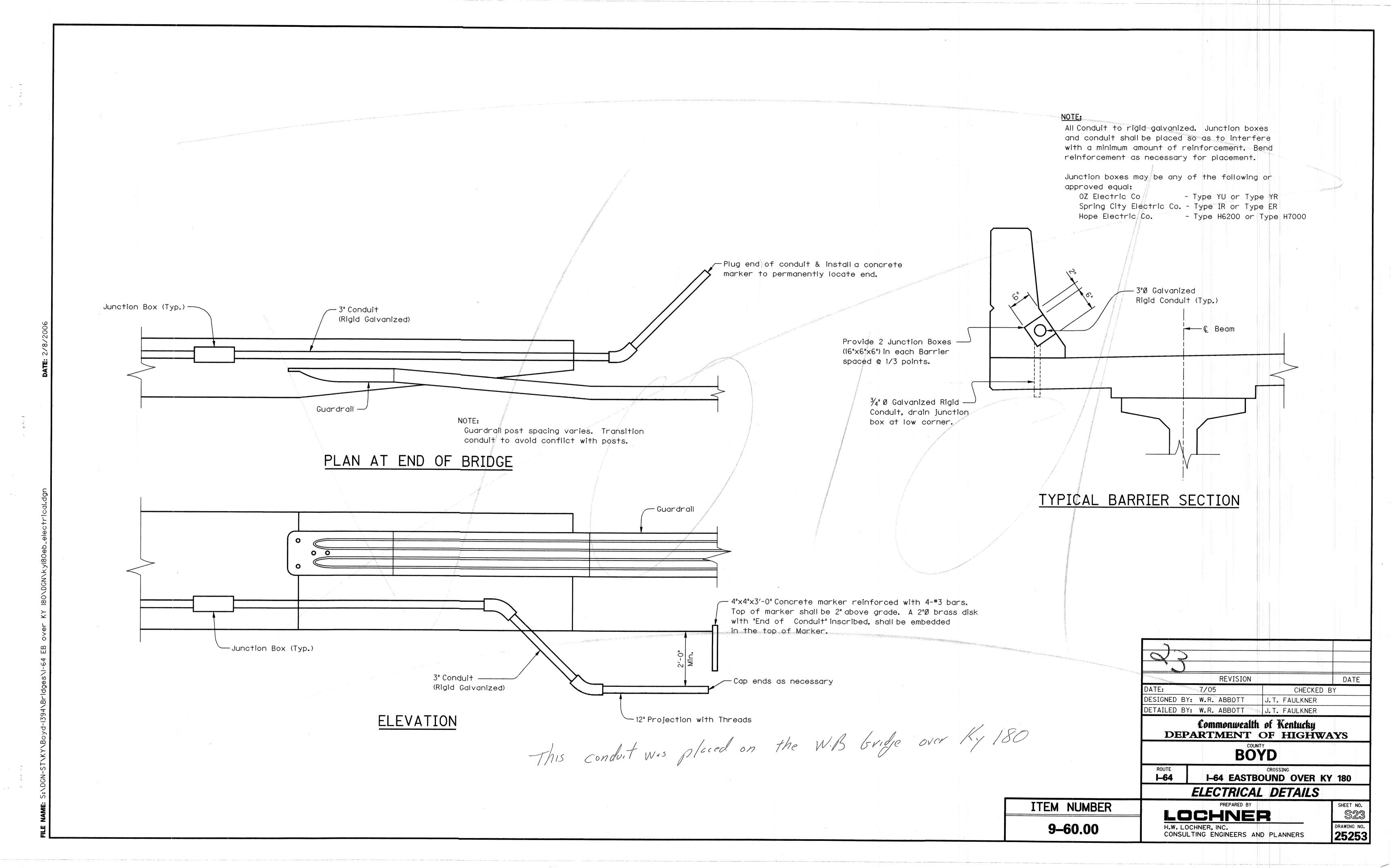
CONSTRUCTION ELEVATIONS

PREPARED BY

LOCHNER

H.W. LOCHNER, INC. CONSULTING ENGINEERS AND PLANNERS

ITEM NUMBER 9-60.00



TRANSPORTATION CABINET DEPARTMENT OF HIGHWAYS BOYD COUNTY INTERSTATE 64 (EASTBOUND) OVER KY 180 Station 201+92.13

	ESTIMATE OF QUANTITIES																						
BID ITEM	CONCRETE CLASS A	CONCRETE CLASS AA	STEEL	STEEL REINFORCEMENT EPOXY COATED	PPC I-BEAM 66" / TYPE 6	FOUNDATION	STRUCTURE EXCAVATION SOLID ROCK	CRUSHED AGGREGATE SLOPE PROTECTION	ARMORED EDGE	STRUCTURE GRANULAR BACKFILL	STEEL PILES HP 14x89	PILE POINTS 14 INCH	TEST PILE	MASONRY COATING	ELECTRICAL								
UNIT	C.Y.	C.Y.	LBS.	LBS.	L.F.	L.S.	C.Y.	TON	L.F.	C.Y.	L.F.	EA.	L.F.	S.Y.	L.S.								
Integral End Bent 1	33.3			5776				126	45	262	227	8	35	59									
Pier 1	115.0		17556	120			76							145									
Semi-Integral Abutment 2	39.4			6105			46		45	265				49				\					
D .																							
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			***************************************			-													<u> </u>				

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								· ·						,								· ·	
Superstructure		458.1		107440	1504.0									1100									
BRIDGE TOTALS	187.7	458.1	17556	119441	1504.0	. 1	122	126	. 90	527	. 227	8	35	1353									



Staking Layout Integral End Bent 1 Pier 1 Semi-Integral Abutment 2 Framing Plan PPC I-Beam, 66", Details Steel Diaphragms Superstructure Curve Offsets Construction Elevations | Electrical Details **SPECIAL NOTES SPECIAL PROVISIONS** 69 Embankment at Bridge End Bent Structures STANDARD DRAWINGS BGX-006-08 | Stencils for Structures BGX-012-02 | Geotechnical Legend BGX-011-04 | Barrier Transition End Drainage BBP-002-04 Bearing Details Elastomeric Bearing Pads for Prestressed Beams Neoprene Expansion Dams and Armored Edges HP 14x89 Steel Pile RBC-001-08 | Guardrail Connector to Bridge End **SPECIFICATIONS** 2004 Standard Specifications for Road and Bridge Construction 2002 AASHTO Standard Specifications for Highway Bridges REVISION 07-05 CHECKED BY DESIGNED BY: W.R. ABBOTT J.T. FAULKNER DETAILED BY: W.R. ABBOTT J.T. FAULKNER Commonwealth of Kentucky DEPARTMENT OF HIGHWAYS **BOYD** I-64 EASTBOUND OVER KY 180 TITLE SHEET PREPARED BY LOCHNER H.W. LOCHNER, INC. CONSULTING ENGINEERS AND PLANNERS

INDEX OF SHEETS

Title Sheet

Layout

General Notes

Bar Bending Details

Foundation Layout

Subsurface Data Sheet

ITEM NUMBER

9-60.00

SHEET NO. S1 25253

DATE

GENERAL NOTES

SPECIFICATIONS:

ALL REFERENCES TO THE STANDARD SPECIFICATIONS ARE TO THE CURRENT EDITION OF THE KENTUCKY DEPARTMENT OF HIGHWAYS STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, WITH CURRENT SUPPLEMENTAL SPECIFICATIONS.

ALL REFERENCES TO AASHTO SPECIFICATIONS ARE TO THE 2002 EDITION AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES.

DESIGN LOAD AND METHOD:

THIS BRIDGE IS DESIGNED FOR HS25 LIVE LOAD OR ALTERNATE MILITARY LOADING, WHICHEVER PRODUCES THE GREATER STRESS. THE HS25 LIVE LOAD IS ARRIVED AT BY INCREASING THE STANDARD HS20-44 TRUCK AND LANE LOADS AS SPECIFIED IN THE AASHTO SPECIFICATIONS BY 25%. ALL REINFORCED CONCRETE MEMBERS ARE DESIGNED BY THE LOAD FACTOR METHOD AS SPECIFIED IN THE CURRENT AASHTO SPECIFICATIONS.

WIND LOAD:

THIS BRIDGE IS DESIGNED FOR A WIND LOAD BASED ON A WIND VELOCITY OF 100 MPH.

MATERIALS DESIGN SPECIFICATION:

FOR CLASS "A" REINFORCED CONCRETE F'C = 3500 PSIF'C = 4000 PSI FOR CLASS "AA" REINFORCED" CONCRETE FOR STEEL REINFORCEMENT FY = 60000 PSI

REINFORCEMENT:

DIMENSIONS SHOWN FROM THE FACE OF CONCRETE TO BARS ARE TO CENTER OF BARS UNLESS OTHERWISE SHOWN. SPACING OF BARS IS FROM CENTER TO CENTER OF BARS. CLEAR DISTANCE TO FACE OF CONCRETE IS 2". UNLESS OTHERWISE NOTED. EPOXY COAT BARS DESIGNATED BY SUFFIX (e) IN ACCORDANCE WITH SECTION 811.10 OF THE STANDARD SPECIFICATIONS. USE STIRRUP BEND DIAMETERS FOR BARS DESIGNATED BY SUFFIX (s) IN A BILL OF REINFORCEMENT.

BEVELED EDGES:

ALL EXPOSED EDGES SHALL BE BEVELED 1/8" UNLESS OTHERWISE SHOWN.

SHOP DRAWINGS:

SUBMIT SHOP DRAWINGS THAT ARE REQUIRED BY THE PLANS AND SPECIFICATIONS DIRECTLY TO THE BRIDGE CONSULTANT. IF ANY CHANGES IN THE DESIGN PLANS ARE PROPOSED BY A FABRICATOR OR SUPPLIER, SUBMIT THOSE CHANGES TO THE BRIDGE CONSULTANT THROUGH THE CONTRACTOR. THE BRIDGE CONSULTANT SHALL PROVIDE THE DIVISION OF BRIDGE DESIGN A COPY OF THE FINAL APPROVED SHOP PLANS.

INCIDENTAL MATERIALS:

THE STRUCTURE IS TO BE COMPLETED IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS. MATERIAL OR LABOR. NOT OTHERWISE SPECIFIED, ARE TO BE CONSIDERED INCIDENTAL TO THE CONTRACT.

DIMENSIONS:

DIMENSIONS ARE FOR A NORMAL TEMPERATURE AT 60 DEGREES FAHRENHEIT. LAYOUT DIMENSIONS ARE HORIZONTAL MEASUREMENTS.

CONCRETE:

CLASS "AA" CONCRETE IS TO BE USED IN THE SUPERSTRUCTURE DECK, PARAPETS, AND DIAPHRAGMS. CLASS "A" CONCRETE IS TO BE USED IN THE SUBSTRUCTURE. PRESTRESSED GIRDER CONCRETE SHALL BE IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS.

PAYMENT FOR PRESTRESSED PRECAST CONCRETE BEAMS:

THE BASIS OF PAYMENT FOR THE PRESTRESSED CONCRETE BEAMS SHALL BE AT THE CONTRACT UNIT PRICE PER LINEAR FOOT OF BEAM, IN ACCORDANCE WITH THE SPECIFICATIONS.

FOUNDATION PRESSURE:

SPREAD FOOTING FOUNDATIONS ARE DESIGNED FOR PRESSURES AS SHOWN IN THE SPREAD FOOTING RECORD ON THE FOUNDATION LAYOUT SHEET. END BENT PILES ARE DESIGNED FOR THE MAXIMUM AXIAL LOAD AS SHOWN IN PILE RECORD ON THE FOUNDATION LAYOUT SHEET.

ELECTRICAL CONDUIT:

THE LUMP SUM BID FOR THIS ITEM SHALL INCLUDE FURNISHING ALL CONDUIT, CONCRETE MARKERS AND OTHER MATERIALS AND LABOR FOR PLACING THESE MATERIALS IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS.

PILING:

PILING SHALL BE DRIVEN TO REFUSAL. TEST PILES SHALL BE DRIVEN WHERE DESIGNATED ON THE PLANS TO DETERMINE THE LENGTH OF PILE REQUIRED. ALL TEST PILES SHALL BE ACCURATELY LOCATED SO THEY MAY BE USED IN THE FINISHED STRUCTURE.

PILE POINTS:

PILE POINTS ARE REQUIRED FOR ALL PILES. THE POINTS SHALL BE THE TYPE FOR KEYING INTO A SLOPING ROCK SURFACE. PILE POINTS SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

PILE HAMMER:

ANY COMMONLY UTILIZED HAMMER ALLOWED BY THE DIVISION OF CONSTRUCTION WILL BE ADEQUATE TO DRIVE THE PILES TO BEDROCK WITHOUT ENCOUNTERING EXCESSIVE BLOW COUNTS AND OVER-STRESSING THE PILES. THE CONTRACTOR SHALL SUBMIT HIS PILE DRIVING SYSTEM TO THE DEPARTMENT FOR APPROVAL PRIOR TO THE INSTALLATION OF THE FIRST PILE. APPROVAL OF THE PILE DRIVING SYSTEM BY THE ENGINEER WILL BE SUBJECT TO SATISFACTORY FIELD PERFORMANCE OF THE PILE DRIVING PROCEDURES.

POURING SEQUENCE:

THE POURING SEQUENCE OF THE SLAB MAY BE CHANGED WITH THE WRITTEN APPROVAL OF THE DESIGNER.

SLOPE PROTECTION:

SLOPE PROTECTION SHALL BE CRUSHED AGGREGATE SLOPE PROTECTION IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS. GEOTEXTILE FABRIC IS TO BE CONSIDERED INCIDENTAL TO CRUSHED AGGREGATE SLOPE PROTECTION.

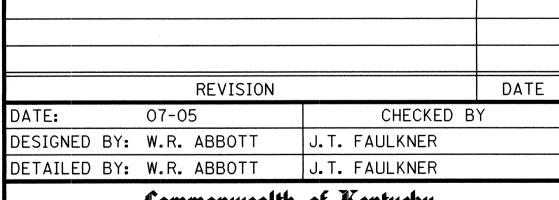
EXISTING ALUMINUM HANDRAIL:

THE EXISTING ALUMINUM HANDRAIL SHALL BE CAREFULLY REMOVED AND DELIVERED. WITHOUT DAMAGE. TO THE STATE MAINTENANCE YARD IN FRANKFORT. PAYMENT FOR THIS WORK SHALL BE INCIDENTAL TO REMOVE EXISTING STRUCTURE, LUMP SUM.

ABBREVIATIONS:

THE FOLLOWING ABBREVIATIONS MAY HAVE BEEN USED IN PREPARATION OF THESE PLANS:

bet.	BETWEEN	MPH	MILES PER HOUR
b.f.	BACK FACE	n.s.	NEAR SIDE
BOF	BOTTOM OF FOOTING	O.D.	OUTSIDE DIAMETER
bot.	BOTTOM	0pp.	OPPOSITE
Brg.	BEARING	PC	POINT OF CURVE
C to C	CENTER TO CENTER	Perp.	PERPENDICULAR
c. e.	CURRENT EDITION	PI	POINT OF INTERSECTION
C.Y.	CUBIC YARD	PPC	PRECAST PRESTRESSED CONCRETE
Chd.	CHORD	PPCDU	PRECAST PRESTRESSED CONCRETE DECK UNIT
CL	CENTER LINE	PSI	POUNDS PER SQUARE INCH
CI.	CLEAR	PT	POINT OF TANGENT
Conc.	CONCRETE	R	RADIUS
Cu.	CUBIC	Rt.	RIGHT
Dwg.	DRAWING	RCBC	REINFORCED CONCRETE BOX CULVERT
e.f.	EACH FACE	RCDG	REINFORCED CONCRETE DECK GIRDER
EI.	ELEVATION	Req'd.	REQUIRED
eq.	EQUAL	RR	RAILROAD
Est.	ESTIMATE	Shld.	SHOULDER
Ext.	EXTERIOR	spa.	SPACES
F to F	FACE TO FACE	Sta.	STATION
f.f.	FRONT FACE	Std.	STANDARD
f.s.	FAR FACE	Str.	STRAIGHT
fr.	FRONT	Tan	TANGENT
ft.	FEET	Thru	THROUGH
I.D.	INSIDE DIAMETER	TOF	TOP OF FOOTING
in.	INCH	Tot.	TOTAL
Int.	INTERIOR	Тур.	TYPICAL
Lt.	LEFT	Vert.	VERTICAL
LBS	LOW BRIDGE SEAT	W.P.	WORKING POINT
LBS.	POUNDS	Yd.	YARD
М	METER		



Commonwealth of Kentucky DEPARTMENT OF HIGHWAYS **BOYD**

I-64 EASTBOUND OVER KY 180

GENERAL NOTES

SHEET NO.

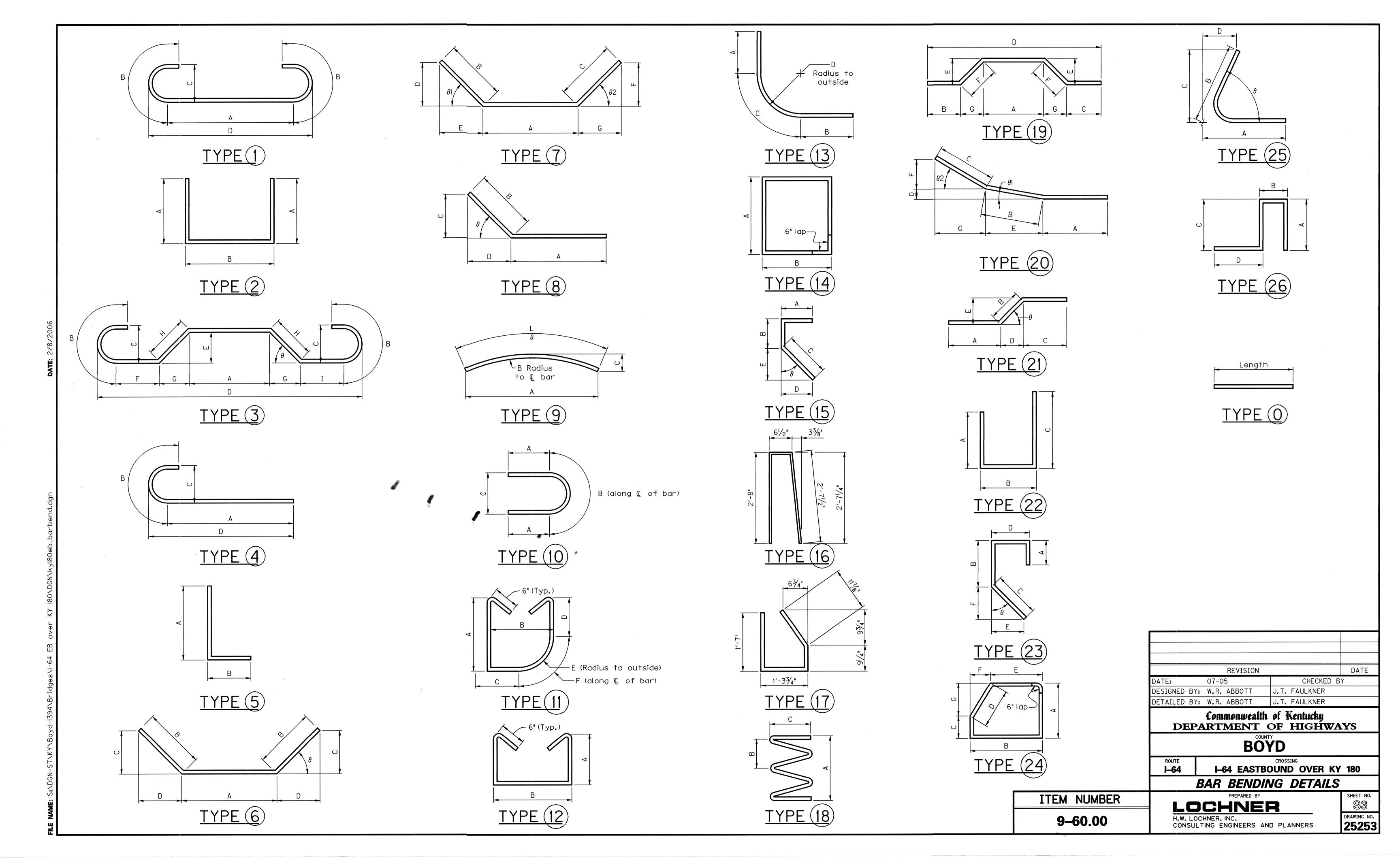
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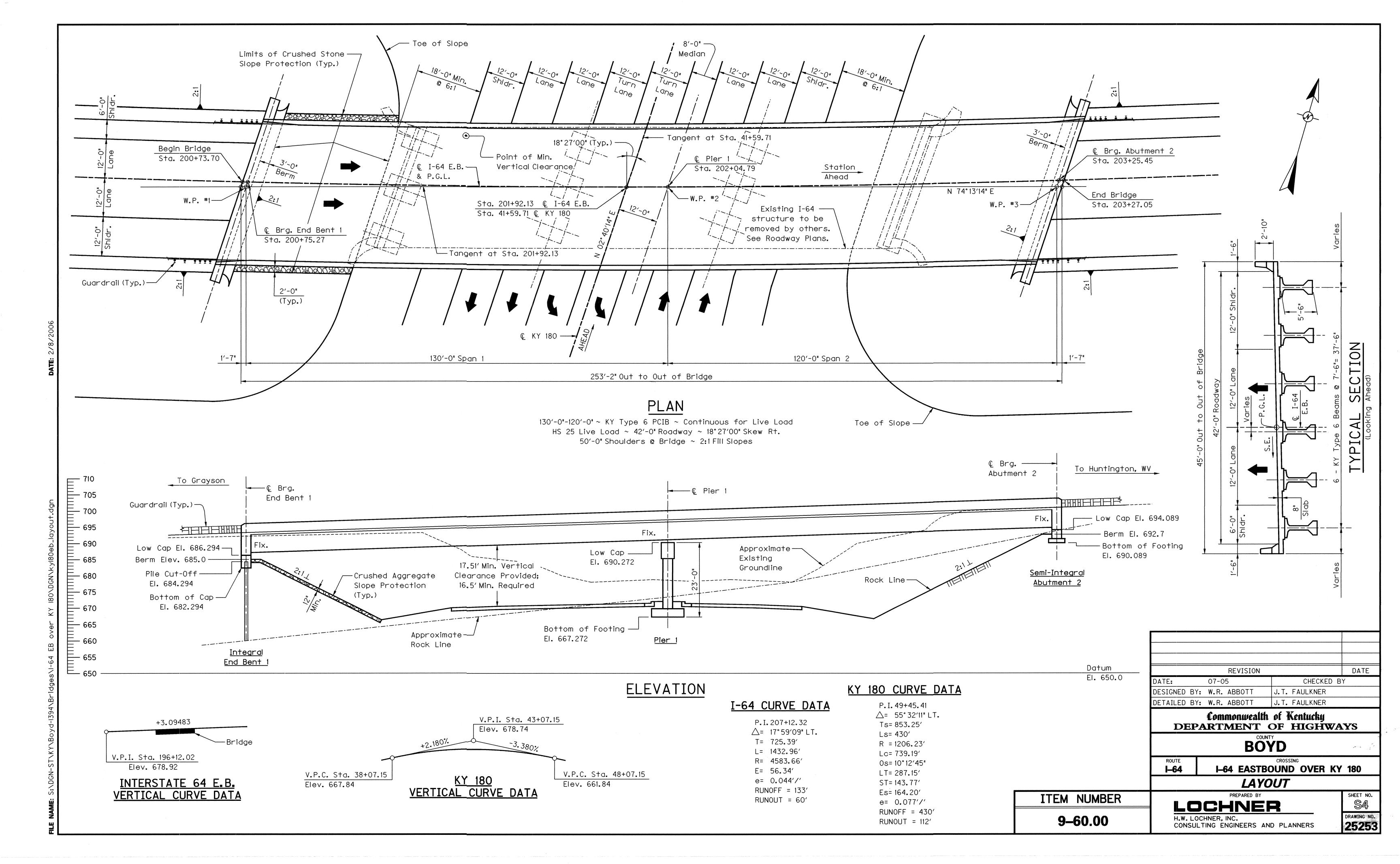
DRAWING NO.

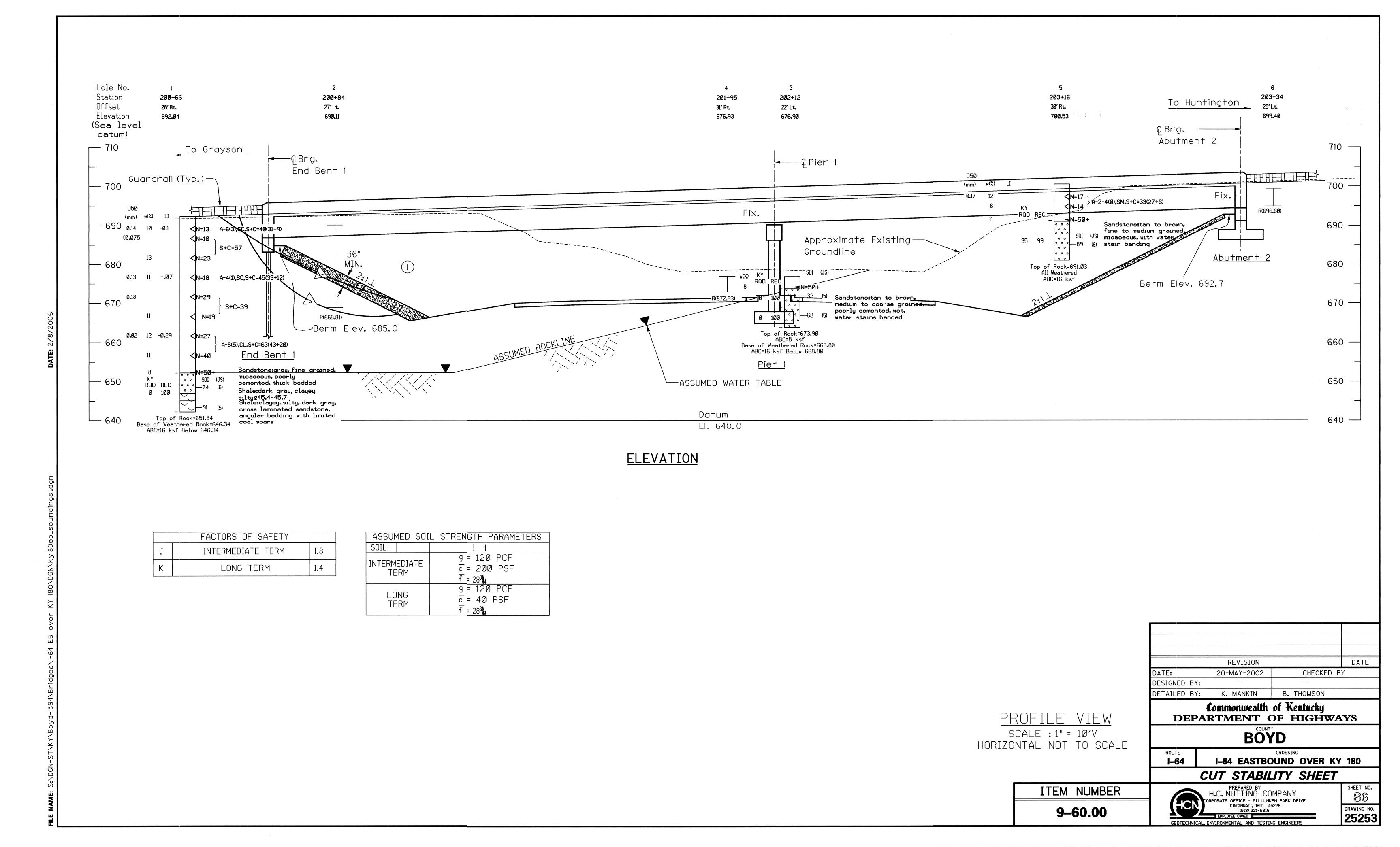
25253

LOCHNER H.W. LOCHNER. INC. CONSULTING ENGINEERS AND PLANNERS

ITEM NUMBER 9-60.00







	PILE REC	ORD FOR	POINT B	EARING F	PILES-END B	ENT 1
Pile No.	Pile Cut-off Elevation (Feet)	Pile Length in Place (Feet)	Point of Pile Elevation as Driven (Feet)	Design Axial Load (Tons)	Required Field Bearing (Tons)	Calculated Field Bearing (Tons)
1 2	684.294 684.294			113 113	180 180	
3 4	684.294 684.294			113 113	180 180	
5 6	684.294 684.294			113 113	180 180	
7	684.294			113	180	
		and the second s				

© Brg. End Bent 1

Sta. 200+75.27

--- W.P. #1

Reinforced Pile Points shall be the type for keying into a sloping rock surface.

6'-03/4"—

35'-0" Test Pile -

Definitions of Terms

PILE CUT-OFF ELEVATION: Elevation of the top of pile in the finished structure.

PILE LENGTH IN PLACE: Actual pile length below the Pile Cut-Off Elevation in the finished structure.

POINT OF PILE ELEVATION AS DRIVEN: Actual point of pile elvevation in the finished structure.

DESIGN AXIAL LOAD: Service load carried by each pile as estimated from structural design calculations.

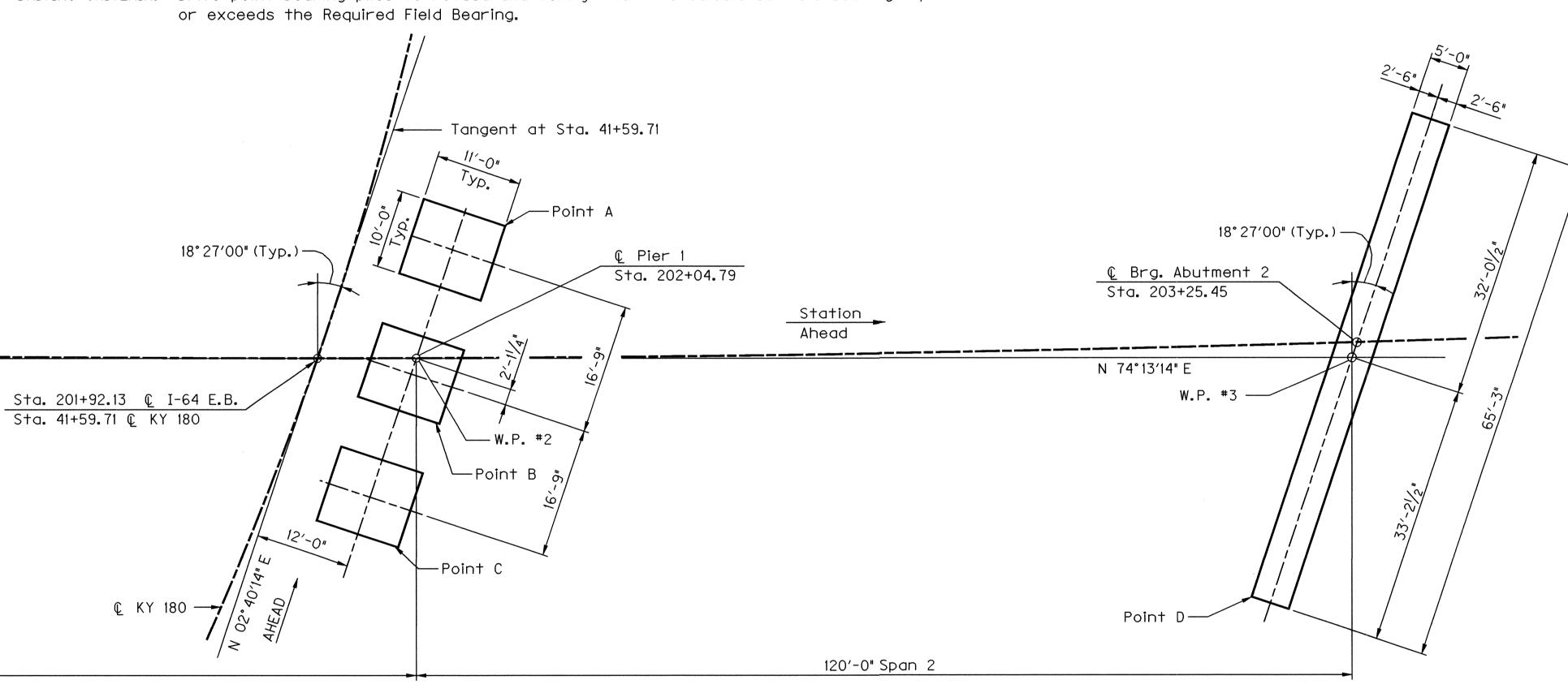
REQUIRED FIELD BEARING: Pile bearing value required to achieve "refusal" for the size of pile used. According to the Division of Construction Guidance Manual, this value is taken as 150 Tons

for 12-inch steel H-Piles and 180 Tons for 14 inch steel H-Piles.

CALCULATED FIELD BEARING: Pile bearing value in place calculated using the appropriate pile driving formula in Section 604.03.07(B) of the Standard Specifications.

<u>Drivina Criteria</u>

DRIVING CRITERIA: Drive point bearing piles to refusal and verify that the Calculated Field Bearing equals



FOUNDATION PLAN

NOTE: After all foundations have been placed, the Project Resident Engineer shall record the bottom of footing elevation "As Built" and shall submit one copy of this sheet with this data to:

Kentucky Transportation Cabinet Director, Division of Bridge Design

Station E3-16-01

200 Mero Street Frankfort, Ky 40622

NOTE: Contrary to the Specifications, bottom of footing elevations shall not be raised.

NOTE: If the spread footing foundation is stepped due to unsuitable material found at the given elevation, the location and elevation of the step shall be shown on this sheet and submitted along with the "As-Built" elevations.

	EAD FOOT CORD PIE			EAD FOOT D ABUTME	
Point	Plan Footing Elevation	As-Built Footing Elevation	Point	Plan Footing Elevation	As-Built Footing Elevation
А	667.272		D	690.089	
В	667.272				
С	667.272				

pressure of 15,512 psf.

130'-0" Span 1

The allowable bearing capacity is 16,000 psf.

© I-64 E.B.—

- Tangent at Sta. 201+92.13

Footing is designed for a maximum | Footing is designed for a maximum pressure of 9,020 psf. The allowable bearing capacity is 16,000 psf.

<u>Field Data</u>

For each pile, the Project Engineer shall record the following on this sheet: Pile Length in Place, Point of Pile Elevation as Driven, and the Calculated Field Bearing. Submit this record to:

> Kentucky Transportation Cabinet Director, Division of Bridge Design Station E3-16-01 200 Mero Street Frankfort, Ky 40622

This pile record does not replace other pile records the Project Engineer is required to keep and submit.

Use HP 14x89 in accordance with BPS-011. c.e.

I	TEM	NU	MBE	R
	9-	· 60 .	.00	

		:	
	REVISION		DATE
DATE:	07-05	CHECKED B	Υ
DESIGNED BY:	W.R. ABBOTT	J.T. FAULKNER	
DETAILED BY:	W.R. ABBOTT	J.T. FAULKNER	
DEPA	Commonwealth RTMENT (of Kentucky OF HIGHWA	YS
	COUNT	Y	

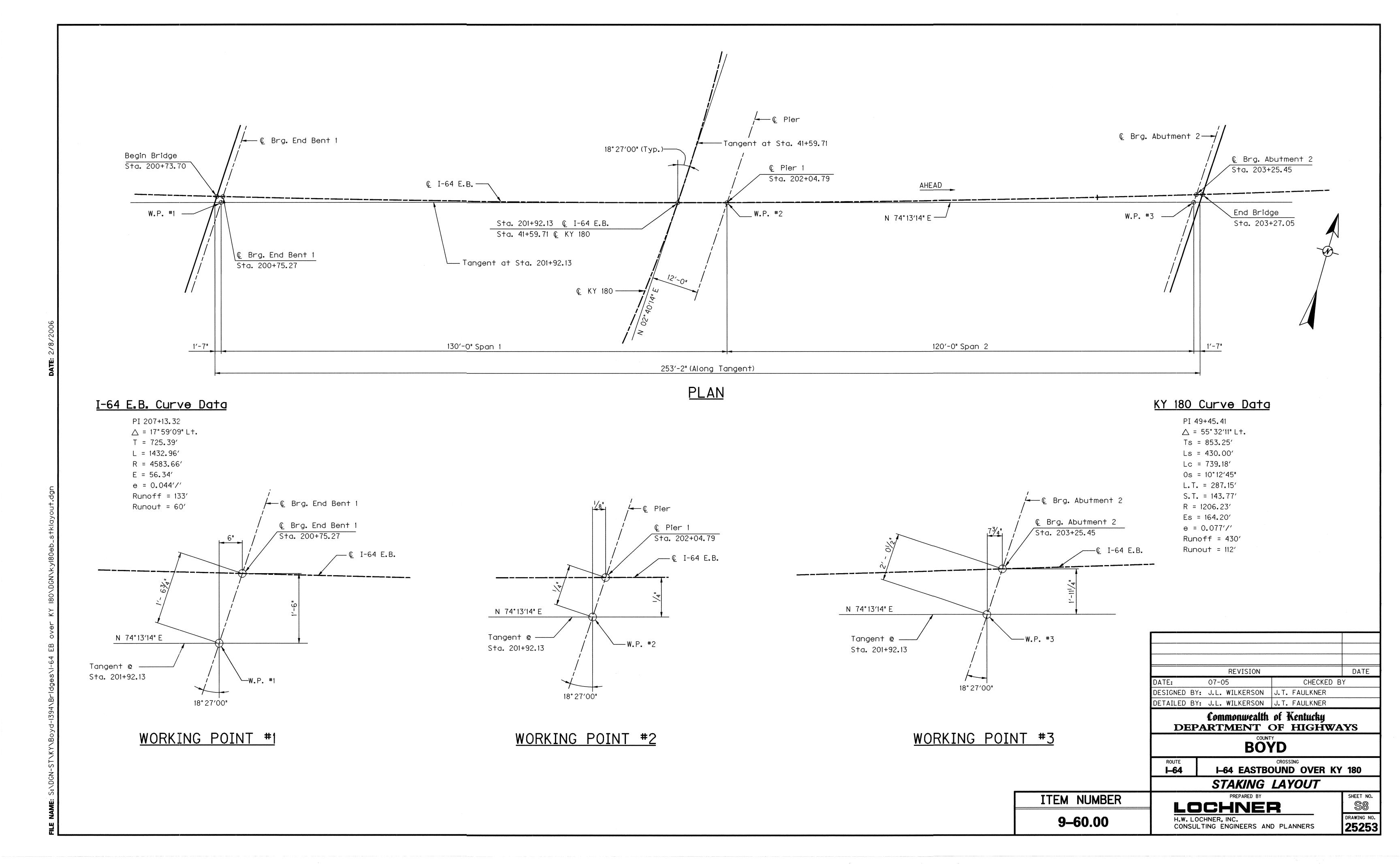
BOYD

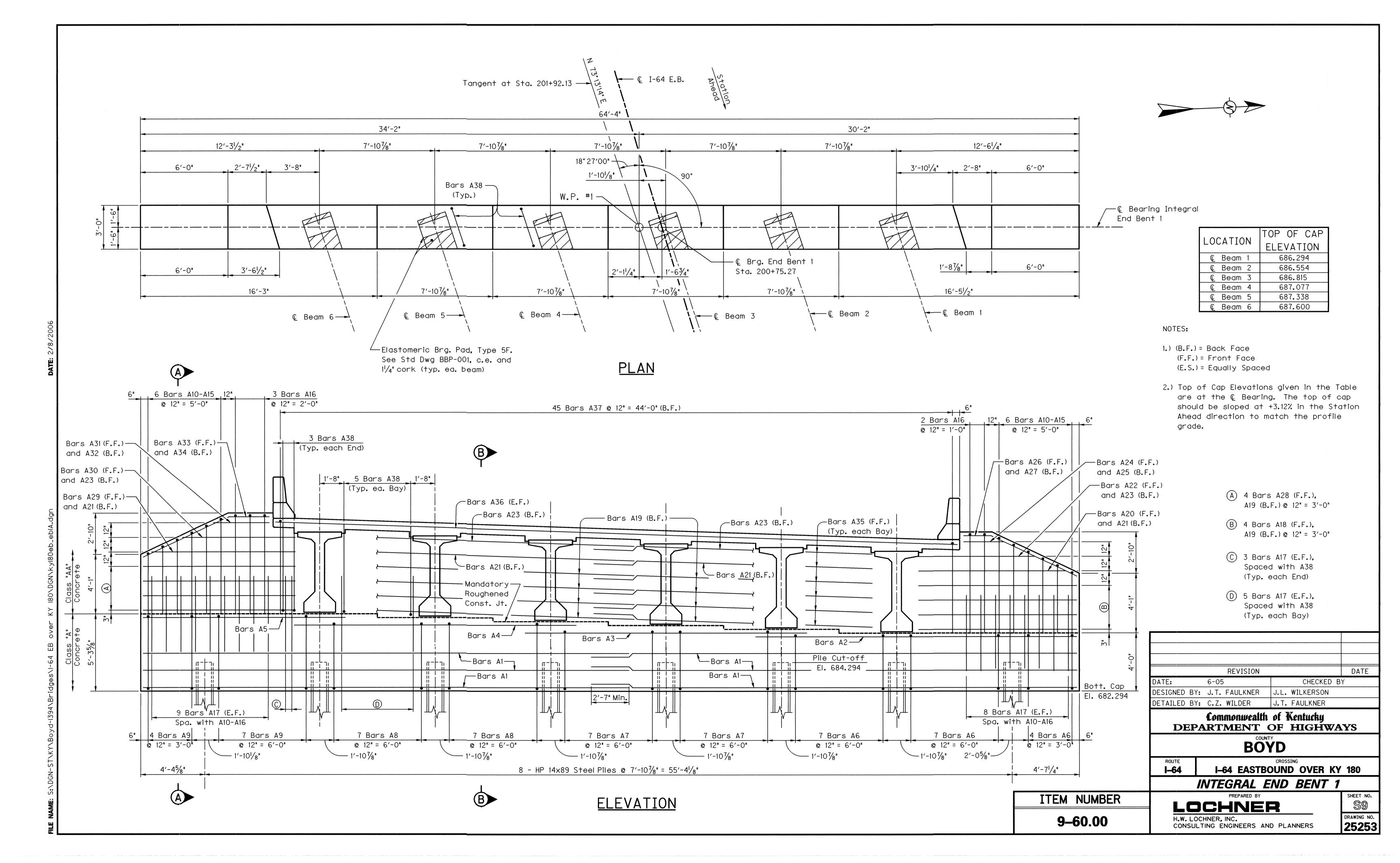
I-64 EASTBOUND OVER KY 180

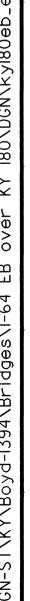
FOUNDATION LAYOUT

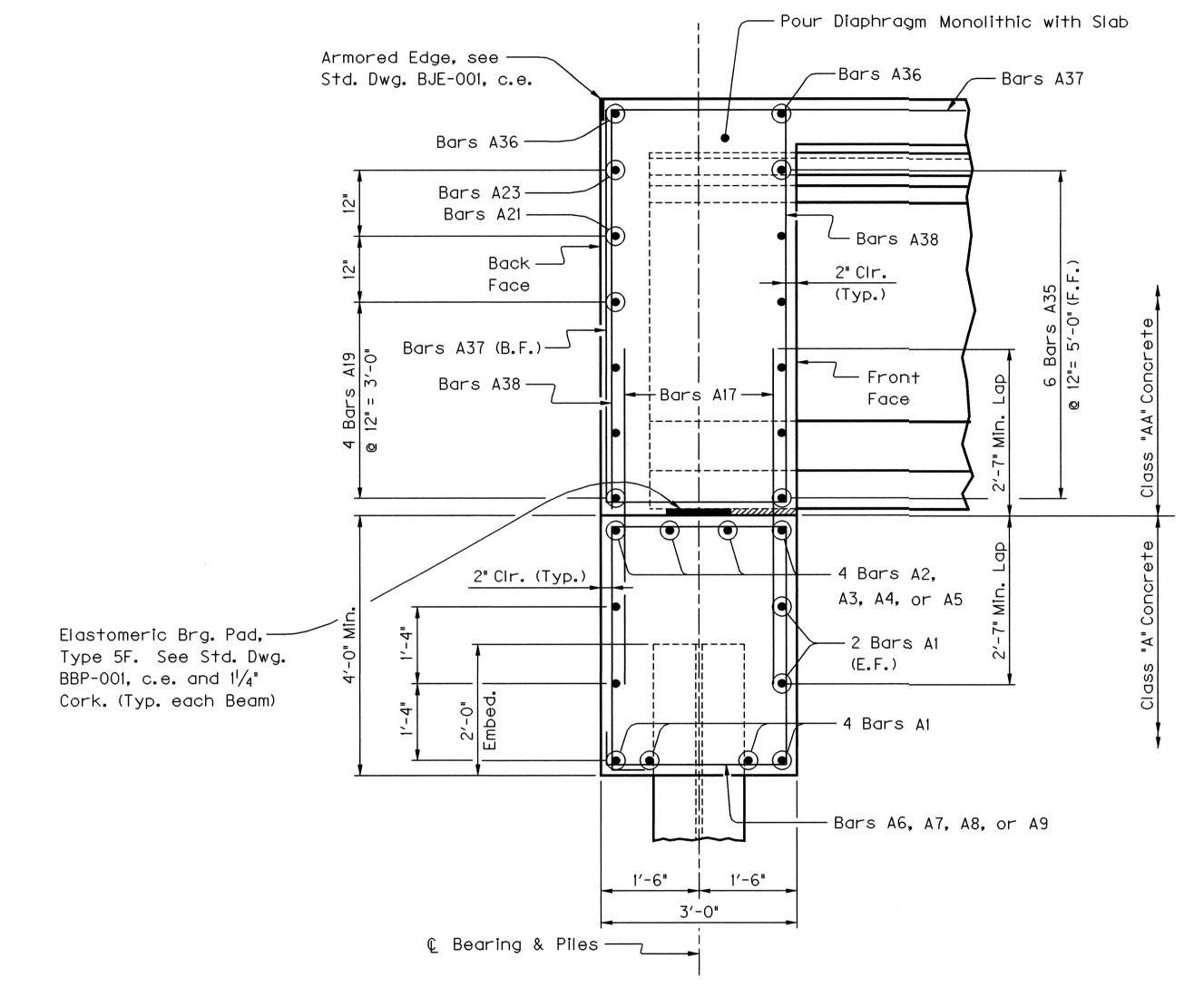
LOCHNER H.W. LOCHNER, INC. CONSULTING ENGINEERS AND PLANNERS

SHEET NO. S7 25253

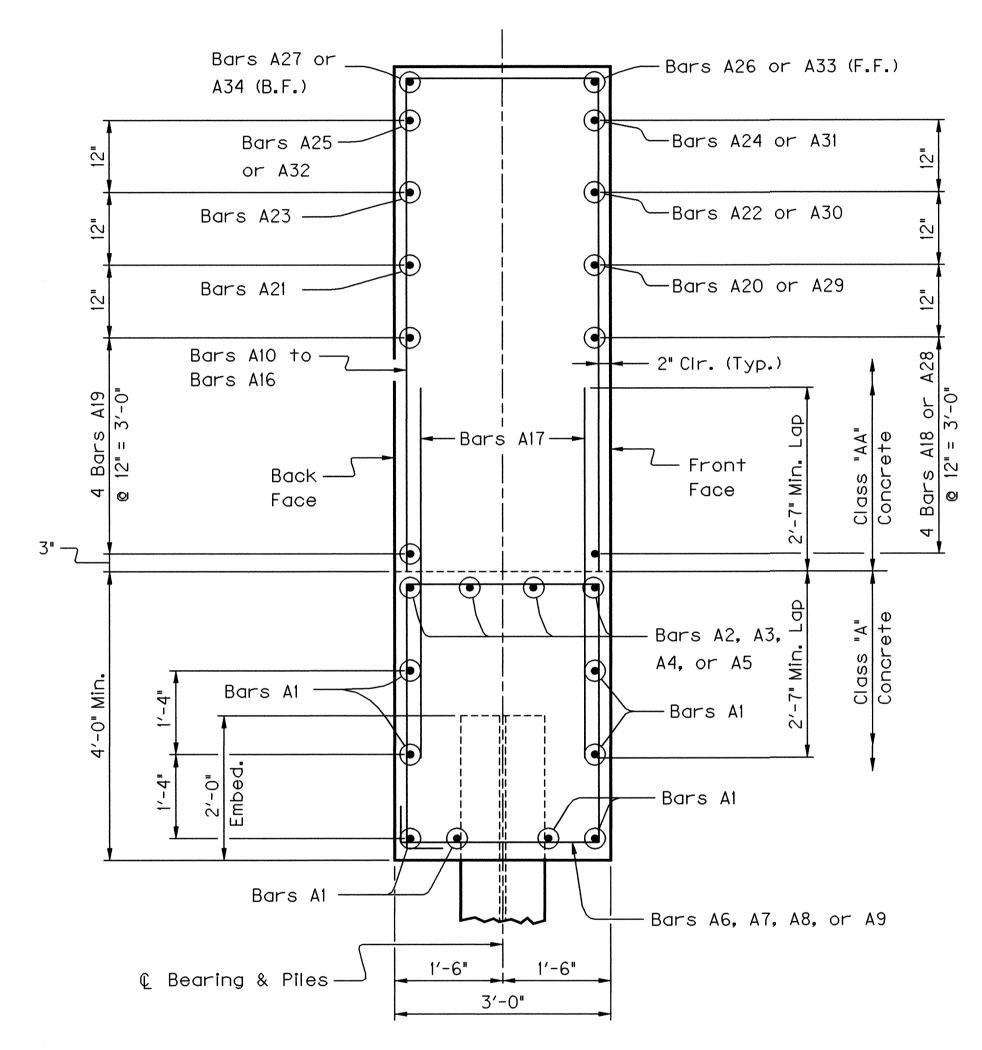








SECTION B-B



SECTION A-A

				LEN	GTH			A		 В		С		D
MARK	TYPE	SIZE	NO.	FT.		LOCATION	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.
Ale	Str	5	16	33	4	Сар								
A2e	Str	5	4	22	6	Cap								
A3e	Str	5	4	21	0	Cap								
A4e	Str	5	4	21	0	Cap								
A5e	Str	5	4	14	0	Cap								
A6es	14	5	18	13	3	Cap	3	7	2	8				
A7es	14	5	14	13	9	Cap	3	10	2	8				
A8es	14	5	14	14	9	Сар	4	4	2	8				
A9es	14	5	11	15	9	Сар	4	10	2	8				
A10es	2	5	2	10	8	Wing	4	01/2	2	8				
Alles	2	5	2	11	8	Wing	4	$\frac{61/2}{2}$	2	8				
Al2es	2	5	2	12	7	Wing	5	0	2	8				
Al3es	2	5	2	13	6	Wing	5	51/2	2	8				
A14es	2	5	2	14	6	Wing	5	111/2	2	8				
Al5es	2	5	2	15	5	Wing	6	5	2	8				
Al6es	2	5	5	15	11	Wing	6	8	2	8				
A17e	Str	5	96	5	6	Wing/Cap								
A18e	Str	5	4	10	1	Wing							İ	
A19e	Str	5	8	33	4	Wing								
	Str		1	9	5	Wing								
	Str		2	32	9	Wing		×				***************************************		
A22e	Str	5	1	7	3	Wing								
A23e	Str		2	30	7	Wing								
A24e	Str		1	2	9	Wing								
A25e	Str	5	1	3	5	Wing								
A26e	8	5	1	8	0	Wing	6	6	1	6	0	75/8	1	41/4
A27e	8	5	1	8	11	Wing	6	6	2	5	1	03/8	2	21/4
A28e	Str	5	4	11	3	Wing						- 78	 	-/-
A29e	Str		1	10	8	Wing								
	Str		1	8	6	Wing								
A31e	Str		 	4	4	Wing				***************************************				
A32e	Str	5	1	3	6	Wing							<u> </u>	
A33e	8	5	1	9	9	Wing	6	6	3	3	t	45/8	2	111/4
A34e	8	5	<u> </u>	8	11	Wing	6	6	2	5	1	03/8	2	21/4
A35e	Str		30	5	3	Diaphragm			<u> </u>		<u> </u>	78		-/4
A36e	Str	5	2	46	9	Slab		6				V		
A37e	5	8	45	16	0	Diaphragm	10	0	6	0			 	
A38es		5	31	14	9	Diaphragm Diaphragm	6	0	2	10				
	 	-	 	├ ं	-	Diapin agin	-	<u> </u>			†		 	<u> </u>

- 1.) Reinforcing Bars designated by suffix (s) shall be considered a stirup for purposes of bend diameters.
- 2.) All Class "AA" Concrete is included in Superstructure Quantities.

	A	. C 707	
DETAILED BY:	C.Z. WILDER	J.T. FAULKNER	
DESIGNED BY:	J.T. FAULKNER	J.L. WILKERSON	
DATE:	06-05	CHECKED B	Υ
	REVISION		DATE

Commonwealth of Kentucky DEPARTMENT OF HIGHWAYS

BOYD

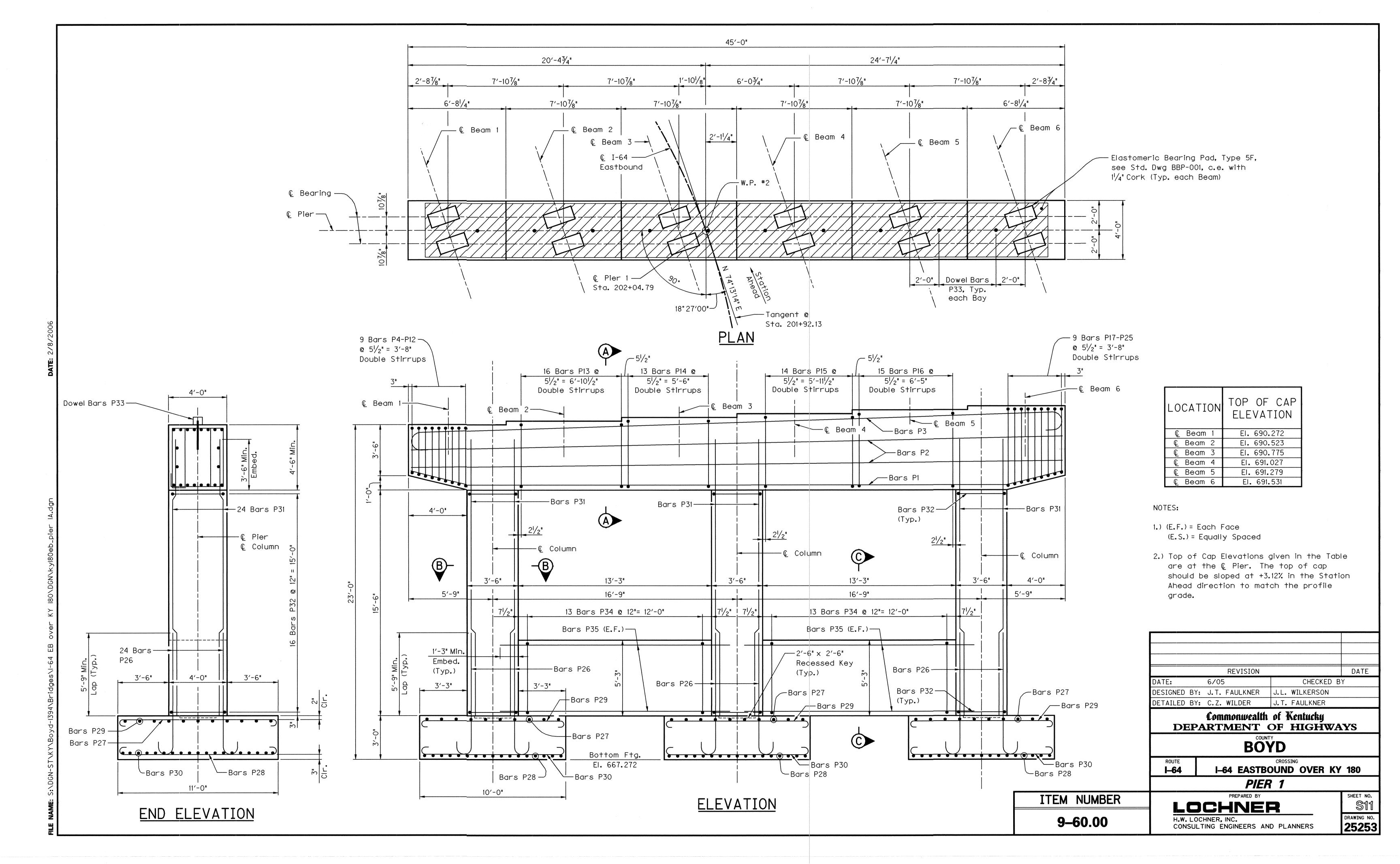
I-64 EASTBOUND OVER KY 180

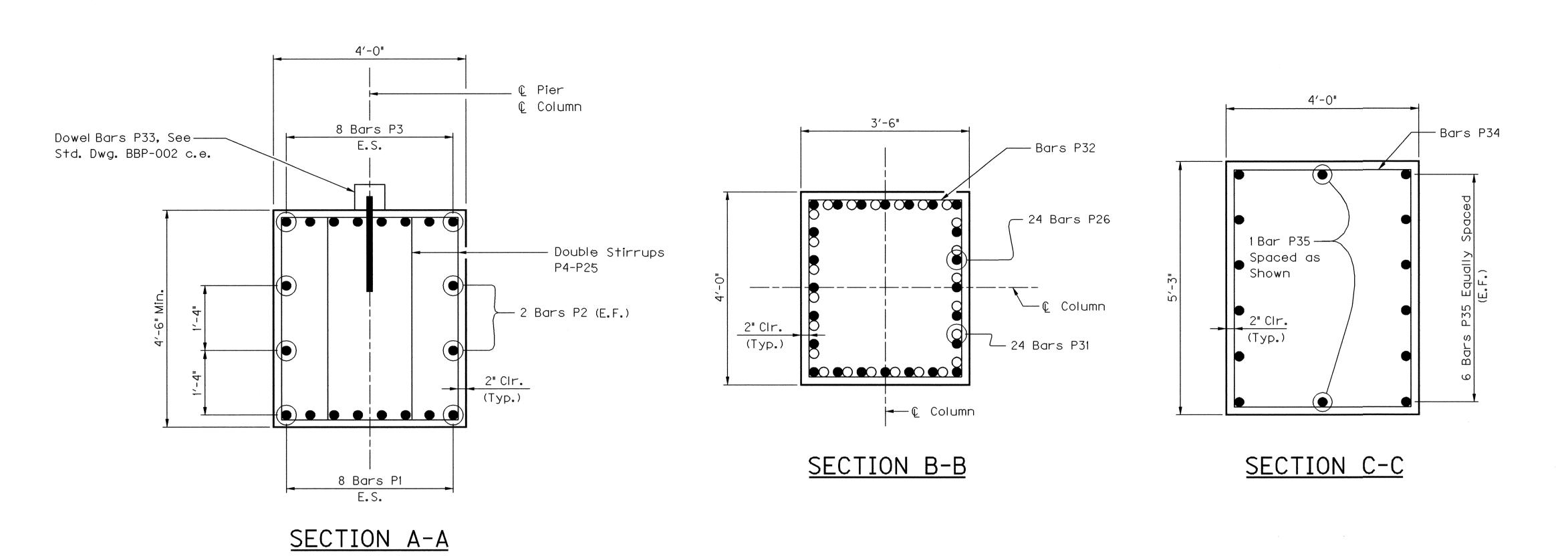
INTEGRAL END BENT 1

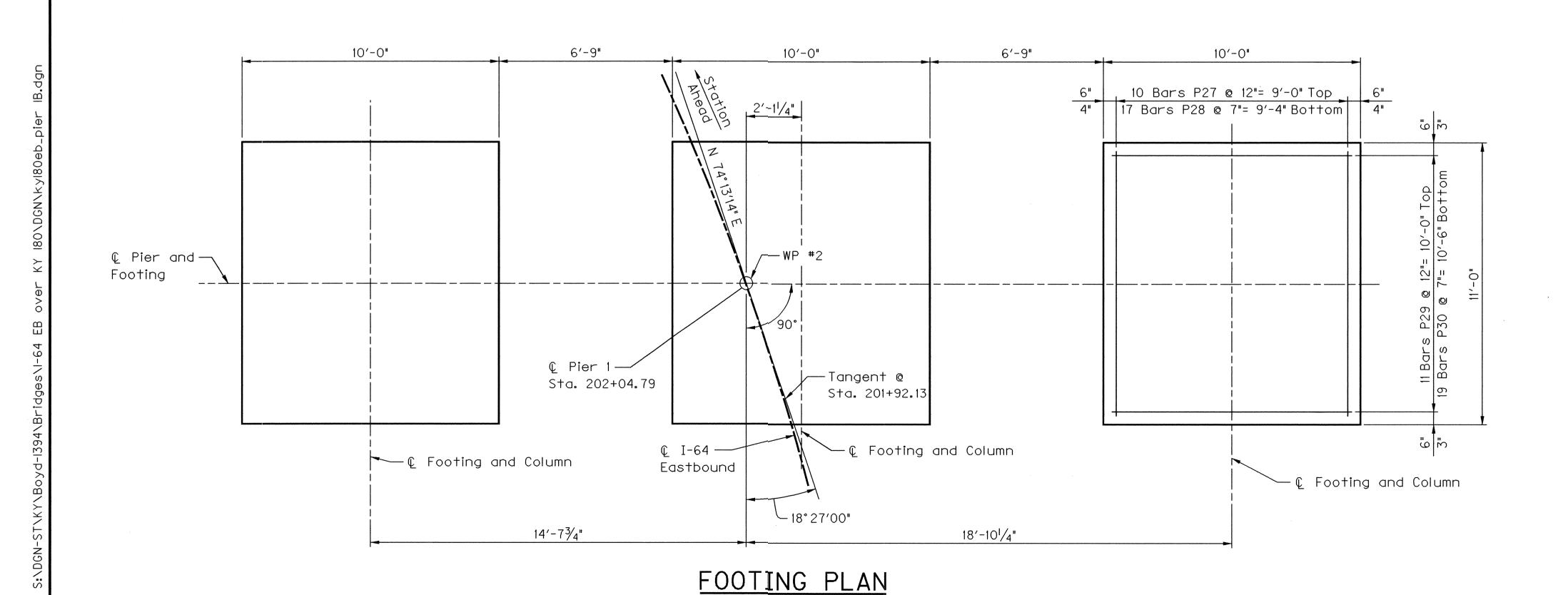
ITEM NUMBER LOCHNER 9-60.00

H.W. LOCHNER, INC. CONSULTING ENGINEERS AND PLANNERS

SHEET NO. \$10 DRAWING NO. 25253







MADK	TVDE	CIZE	NO	LEN	GTH	LOCATION		А		В		С		D
MARK	TTPE	217F	NO.	FT.	IN.	LOCATION	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.
P1	6	9	8	44	9	Cap	36	11	3	11	0	113/8	3	95/
P2	Str	5	4	44	8	Сар								
Р3	1	11	8	48	3	Cap	43	5	2	5	1	23/4	44	73/
P4s	14	5	2	12	8	Cap	3	$2\frac{1}{2}$	2	9				
P5s	14	5	2	12	11	Cap	3	4	2	9				
P6s	14	5	2	13	2	Cap	3	$5\frac{1}{2}$	2	9				
P7s	14	5	2	13	5	Cap	3	7	2	9				
P8s	14	5	2	13	7	Сар	3	8	2	9				
P9s	14	5	2	13	10	Cap	3	91/2	2	9				
P10s	14	5	2	14	1	Cap	3	11	2	9				
Plis	14	5	2	14	4	Сар	4	01/2	2	9				
P12s	14	5	2	14	7	Сар	4	2	2	9				
P13s	14	5	32	15	1	Сар	4	5	2	9				
P14s	14	5	26	15	7	Сар	4	8	2	9				
P15s	14	5	28	16	1	Сар	4	11	2	9				
P16s	14	5	30	16	7	Cap	5	2	2	9				
P17s	14	5	2	17	1	Cap	5	5	2	9				
P18s	14	5	2	16	10	Cap	5	$3\frac{1}{2}$	2	9				
P19s	14	5	2	16	7	Cap	5	2	2	9				
P20s		5	2	16	4	Сар	5	01/2	2	9		·.		
P21s		5	2	16	2	Сар	4	111/2	2	9				
P22s		5	2	15	11	Cap	4	10	2	9		`		
P23s		5	2	15	8	Cap	4	81/2	2	9				
P24s		5	2	15	5	Cap	4	7	2	9				
P25s		5	2	15	3	Cap	4	6	2	9				
P26	4	9	72	10	11	Column/Ftg.	9	0	1	11	0	113/4	9	57
P27	1	5	30	11	11	Footing	10	3	0	10	0	5	10	8
P28	1	6	51	12	2	Footing	10	2	1	0	0	6	10	8
P29	1	5	33	10	11	Footing	9	3	0	10	0	5	9	8
P30	1	6	57	11	2	Footing	9	2	1	0	0	6	9	8
	Str	9	72	19	0	Column				***************************************				
P32s		5	48	14	5	Column	3	8	3	2				
P33e		*	10	2	0	Cap			<u> </u>		†			
P34s		5	26	17	11	Crashwall	4	11	3	8	†			
	Str	5	28	15	9	Crashwall	 				<u> </u>			

^{*} $1\frac{1}{2}$ "Ø Smooth Round Bar, may be commercial grade steel.

- Reinforcing bars designated by suffix (s) shall be considered a stirrup for purposes of bend diameter.
- 2) Reinforcing bars designated by suffix (e) shall be epoxy coated.

REVI	SION	DATE
DATE: 6/05	HECKED BY	
DESIGNED BY: J.T. FAUL	KNER J.L. WILKE	ERSON
DETAILED BY: C.Z. WILD	ER J.T. FAUL	KNER
Commoni DEPARTME	vealth of Kentu NT OF HIC	
	BOYD	4

I-64 EASTBOUND OVER KY 180

PIER 1

ITEM NUMBER

9-60.00

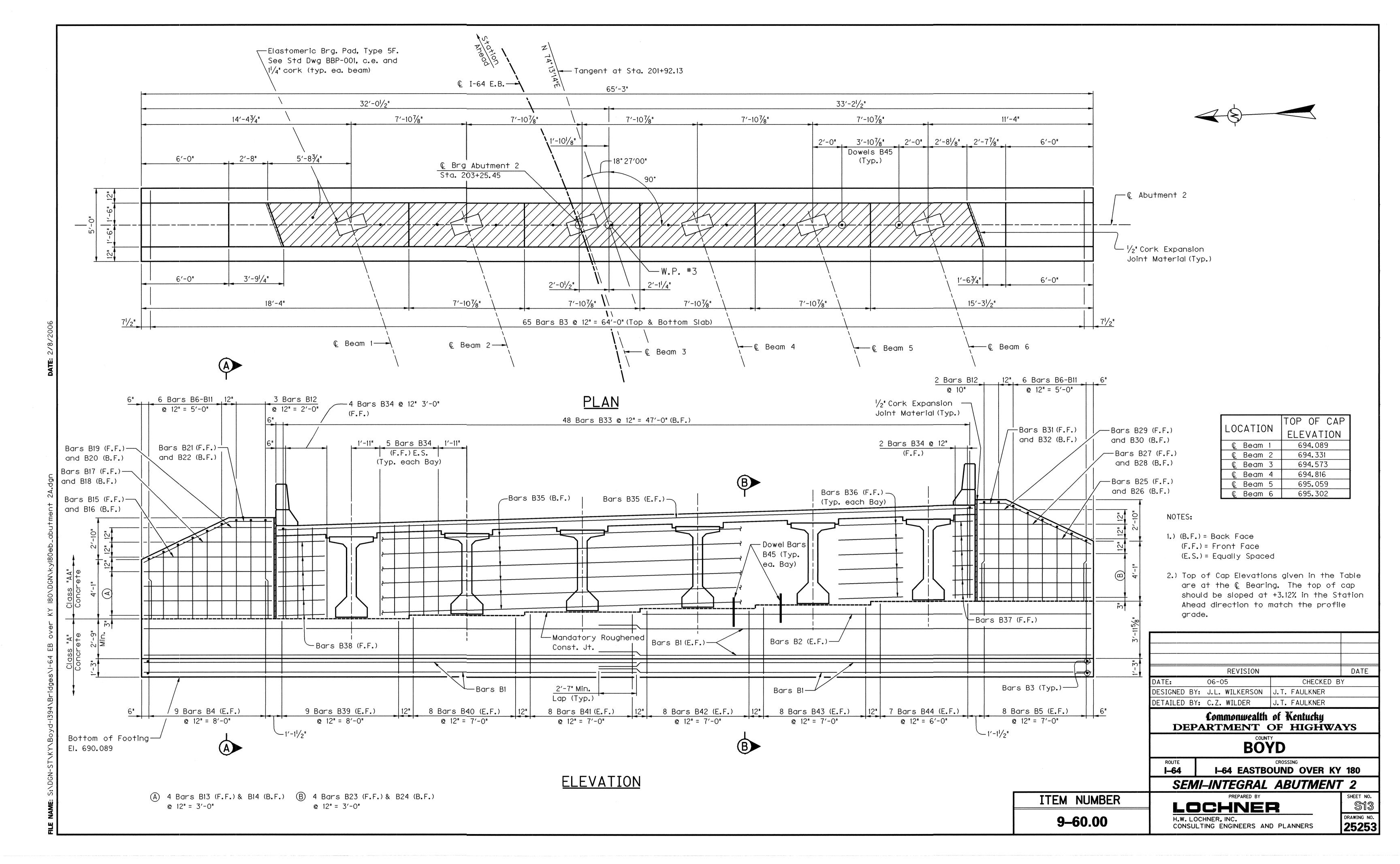
LOCHNER

H.W. LOCHNER, INC.

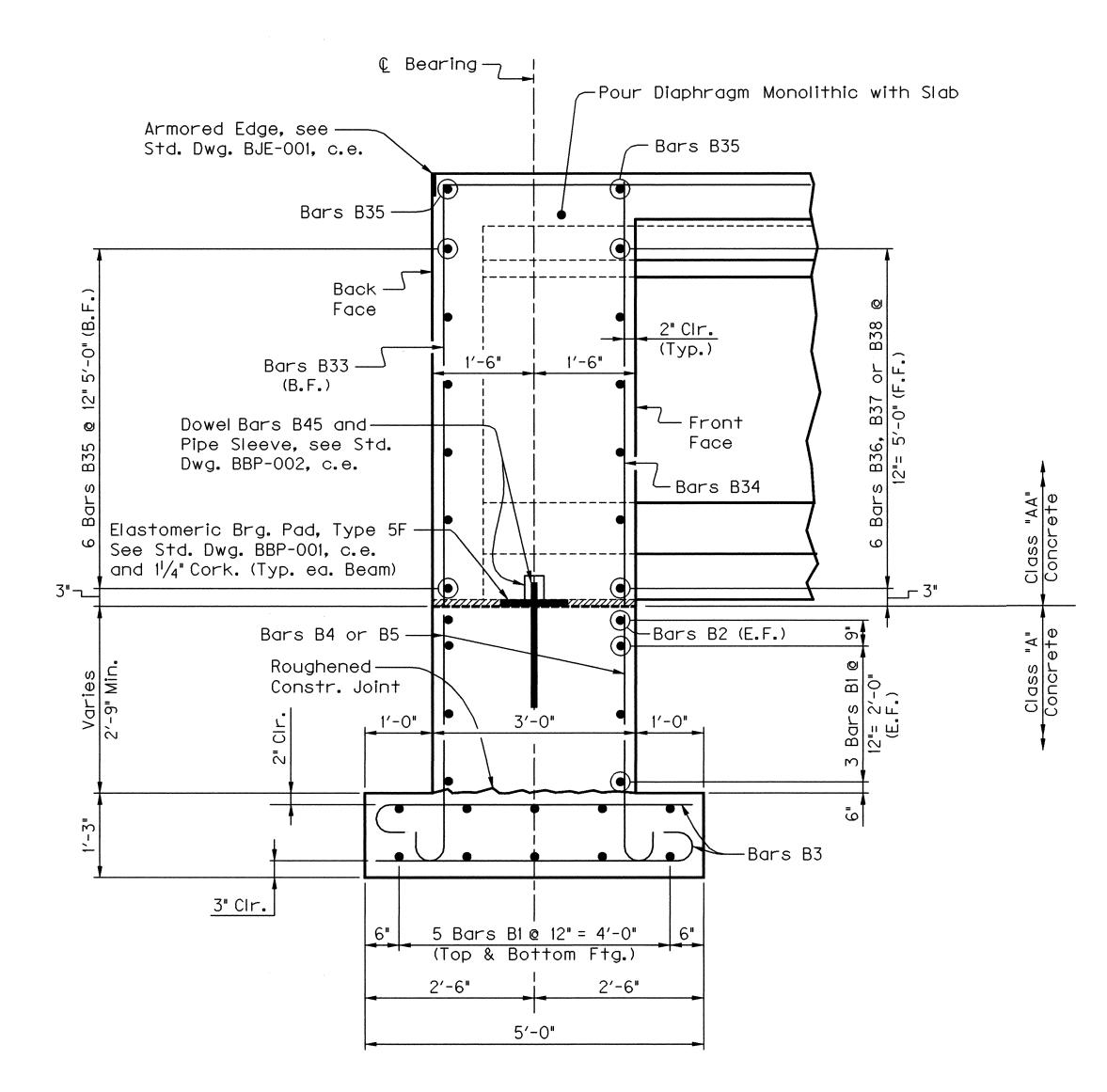
CONSULTING ENGINEERS AND PLANNERS

SHEET NO.
\$12

DRAWING NO.
25253







SECTION B-B

- 1.) Reinforcing Bars designated by suffix (s) shall be considered a stirup for purposes of bend diameters.
- Quantities.

	· · · · · · · · · · · · · · · · · · ·		
	REVISION		DATE
DATE:	06-05	CHECKED B	Υ
DESIGNED BY:	J.L. WILKERSON	J.T. FAULKNER	
DETAILED BY:	C.Z. WILDER	J.T. FAULKNER	

BILL OF REINFORCEMENT

FT. IN. FT. IN. FT. IN. FT. IN.

4 | 5 | 0 | 10 | 0 | 5 | 4 | 7 |

6 4 0 10 0 5 6 6 1/2

7 | 7 | 0 | 10 | 0 | 5 | 7 | 91/2

6 6 3 5 1 53/8 3 11/8

| 6 | 6 | 2 | 5 | 1 | 03/8 | 2 | 21/₄

 $6 \ 6 \ 1 \ 4 \ 0 \ 6\frac{3}{4} \ 1$

10 0 6 0

6 6 2 5 1 0³/₈ 2 2¹/₄

| 3 | 4 | 0 | 10 | 0 | 5 | 3 | 61/2 3 7 0 10 0 5 3 91/2

3 10 0 10 0 5 4 01/2

4 1 0 10 0 5 4 31/2

4 4 0 10 0 5 4 6 1/2

4 7 0 10 0 5 4 91/2

4 01/2 2 8

4 6 1/2 2 8

5 0 2 8 $5 | 5|/_2 | 2 | 8$

 $5 | 11 \frac{1}{2} | 2 | 8$

6 5 2 8

6 8 2 8

LOCATION

Cap/Footing

Footing

Cap/Footing

Cap/Footing

Wing

Diaphragm

Diaphragm

Diaphragm

Diaphragm

Diaphragm

Diaphragm

Cap/Footing

Cap/Footing

Cap/Footing

Cap/Footing

Cap/Footing

Cap/Footing

* $1\frac{1}{2}$ " Diameter Smooth Round Bar, may be Commercial Grade Steel

MARK TYPE SIZE NO. LENGTH

| Bie | Str | 5 | 32 | 33 | 9

B2e Str 5 2 30 6 B3e 4 5 130 5 3

B4e | 4 | 5 | 18 | 7 | 2

B5e 4 5 16 8 5

B6es 2 5 2 10 8 B7es 2 5 2 11 8

B8es 2 | 5 | 2 | 12 | 7

|B9es| 2 | 5 | 2 | 13 | 6

B10es 2 5 2 14 6

Biles 2 5 2 15 5

B12es 2 | 5 | 5 | 15 | 11

B13e Str 5 | 4 | 9 | 4

B14e Str 5 | 4 | 8 | 4

B15e Str 5 | 1 | 8 | 8

B16e Str 5 | 1 | 7 | 8

B17e Str 5 | 1 | 6 | 6

B18e Str 5 1 5 6

B19e Str 5 | 1 | 4 | 5

B20e Str 5 1 3 5

|B21e | 8 | 5 | 1 | 9 | 11

B22e 8 5 1 8 11

B23e Str 5 4 7 2

B24e Str 5 | 4 | 8 | 3

B25e Str 5 1 6 7

B26e Str 5 | 1 | 7 | 8

B27e Str 5 1 4 5

B28e Str 5 | 1 | 5 | 6

B29e Str 5 | 1 | 2 | 4

B30e Str 5 | 1 | 3 | 5

B31e 8 5 1 7 10

B32e 8 5 1 8 11

B33e 5 8 48 16 0

B34e Str 5 31 6 0

B35e|Str| 5 | 8 | 47 | 7

B36e Str 5 | 30 | 5 | 3

B37e Str 5 6 1 9

B38e Str 5 6 3 8

B39e | 4 | 5 | 18 | 4 | 2 |

B40e | 4 | 5 | 16 | 4 | 5 |

B41e 4 5 16 4 8

B42e 4 5 16 4 11

B43e 4 5 16 5 2

B44e 4 5 14 5 5

9-60.00

B45e Str * 10 2 0 Cap/Diaphragm

Commonwealth of Kentucky DEPARTMENT OF HIGHWAYS

BOYD

I-64 EASTBOUND OVER KY 180

SEMI-INTEGRAL ABUTMENT 2

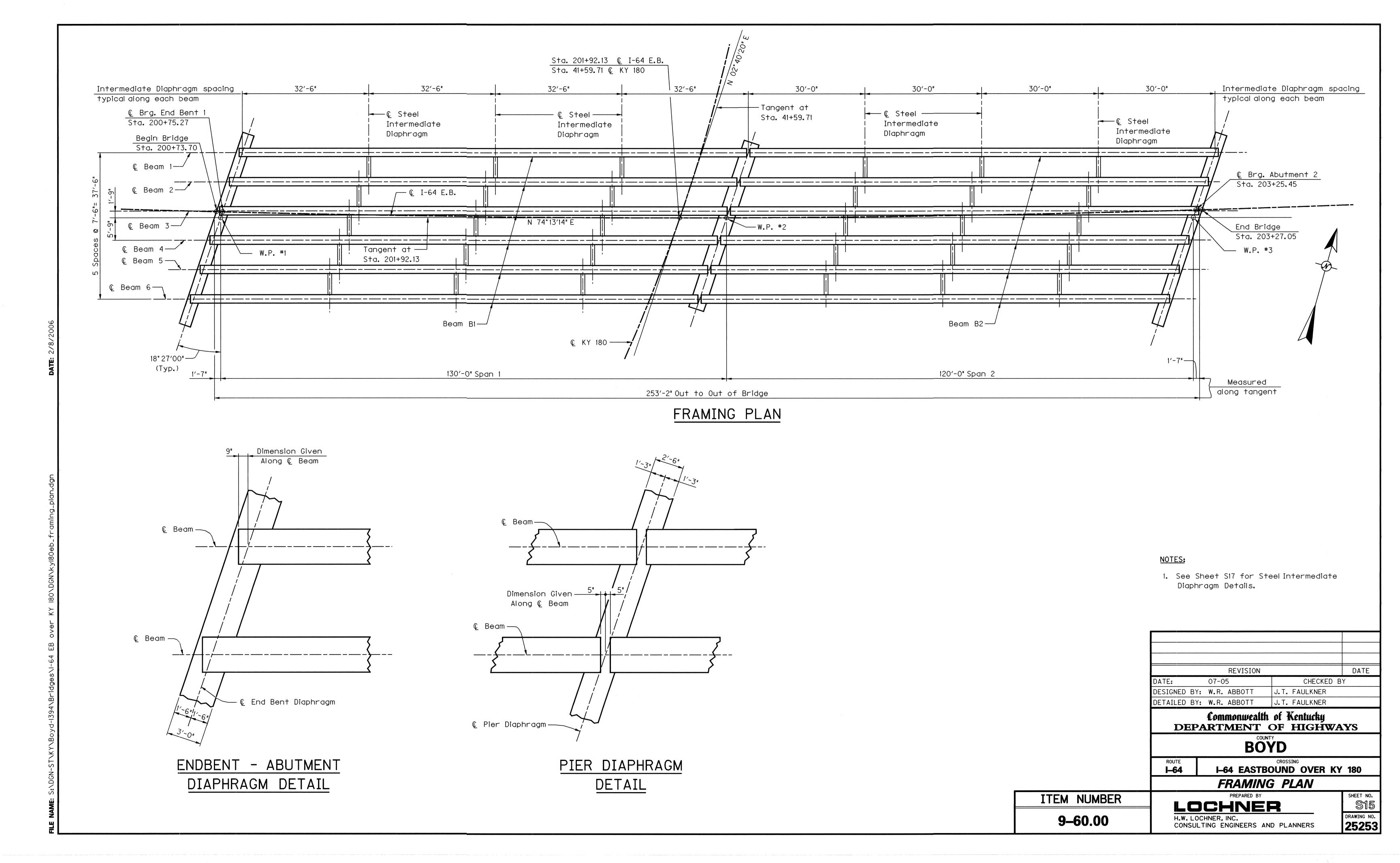
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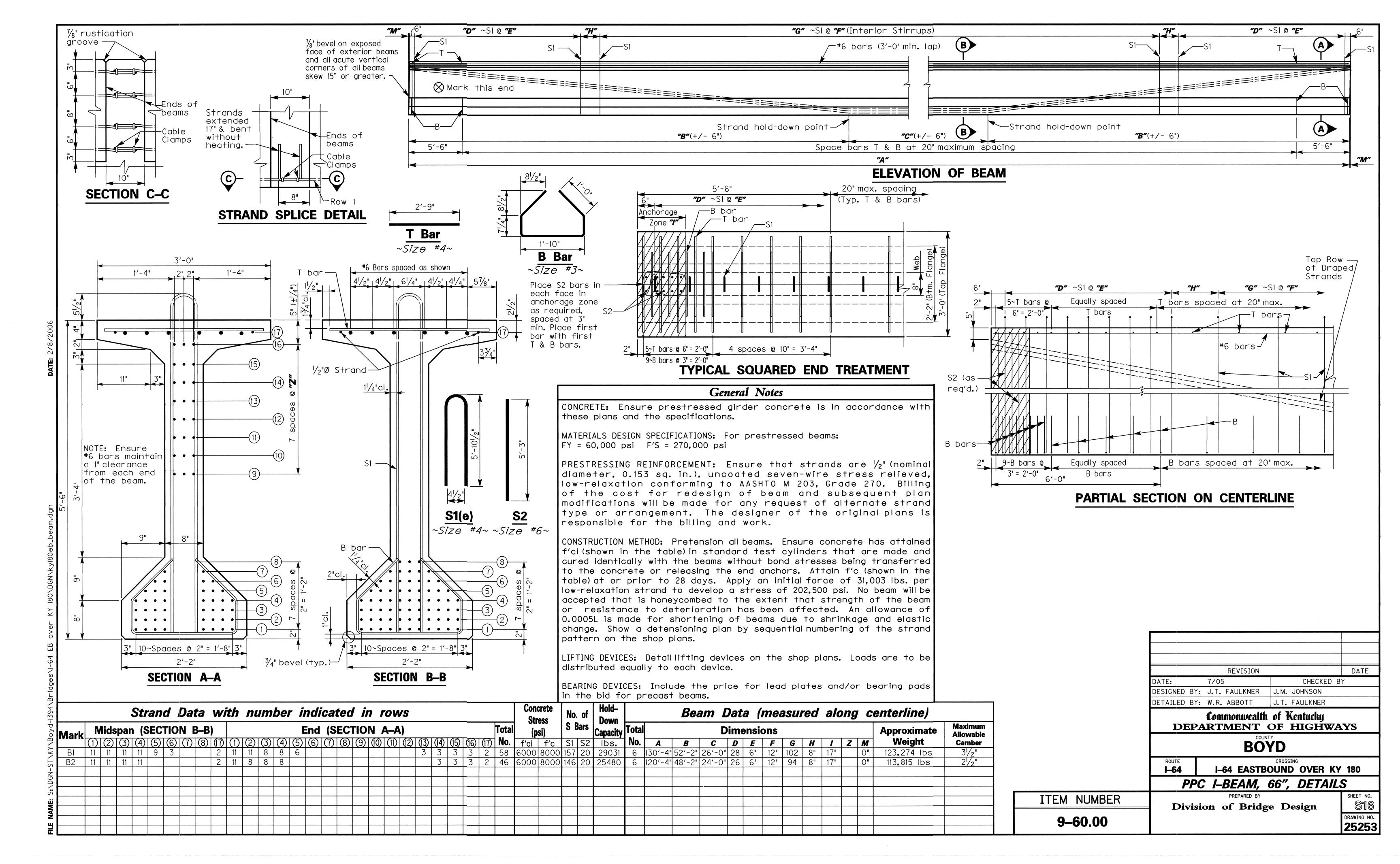
LOCHNER H.W. LOCHNER, INC. CONSULTING ENGINEERS AND PLANNERS

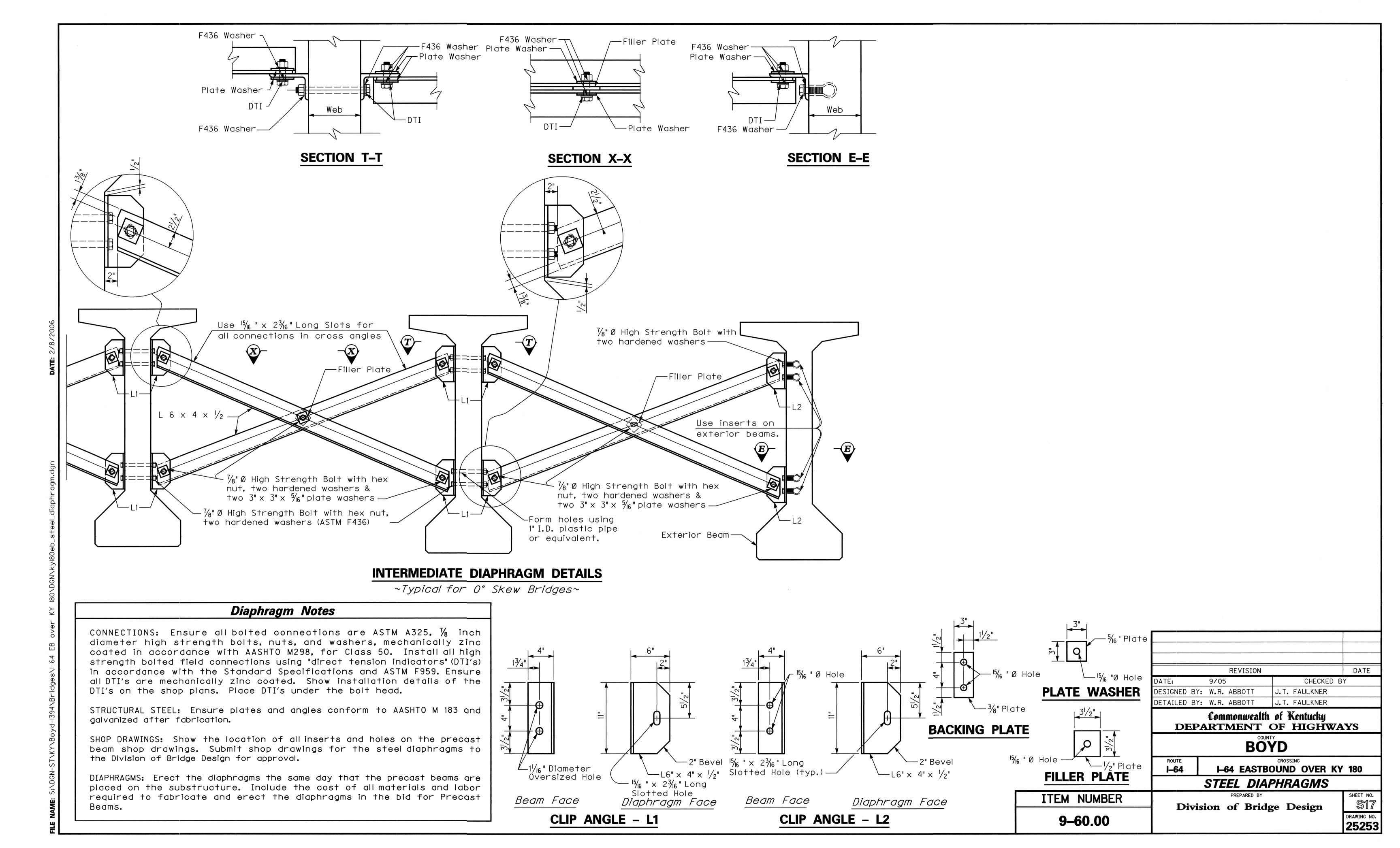
\$14 DRAWING NO. 25253

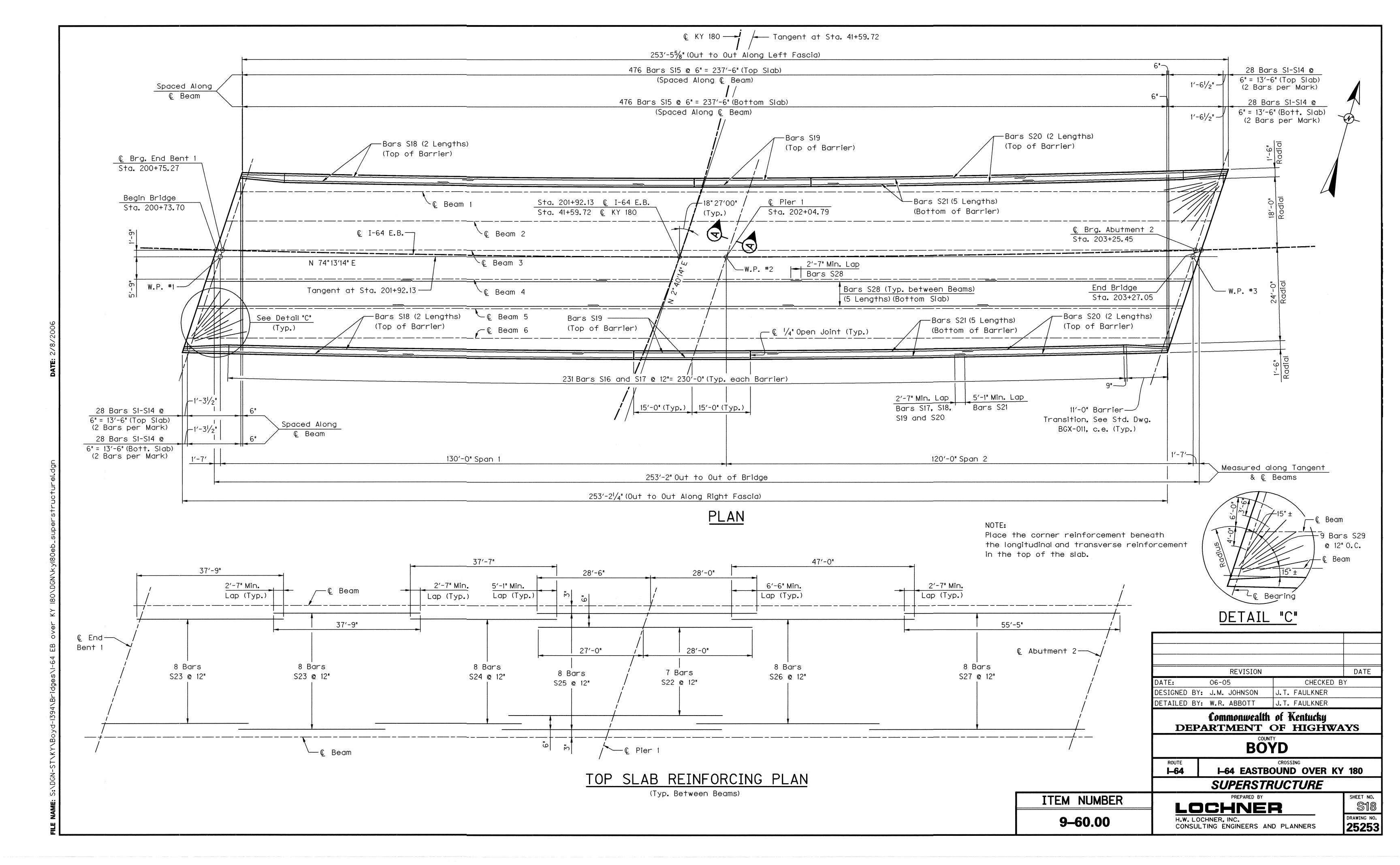
SHEET NO.

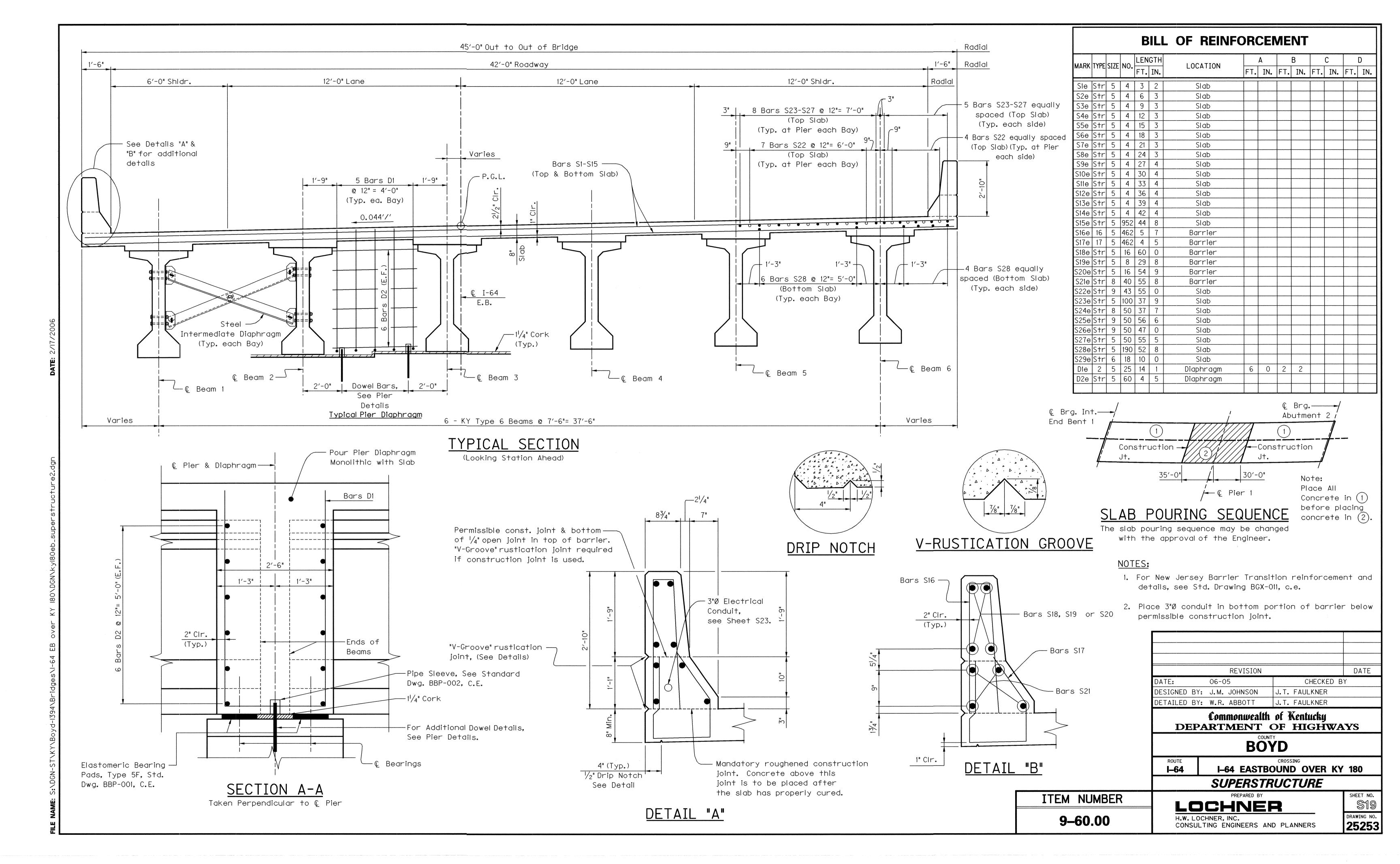
2.) All Class "AA" Concrete is included in Superstructure

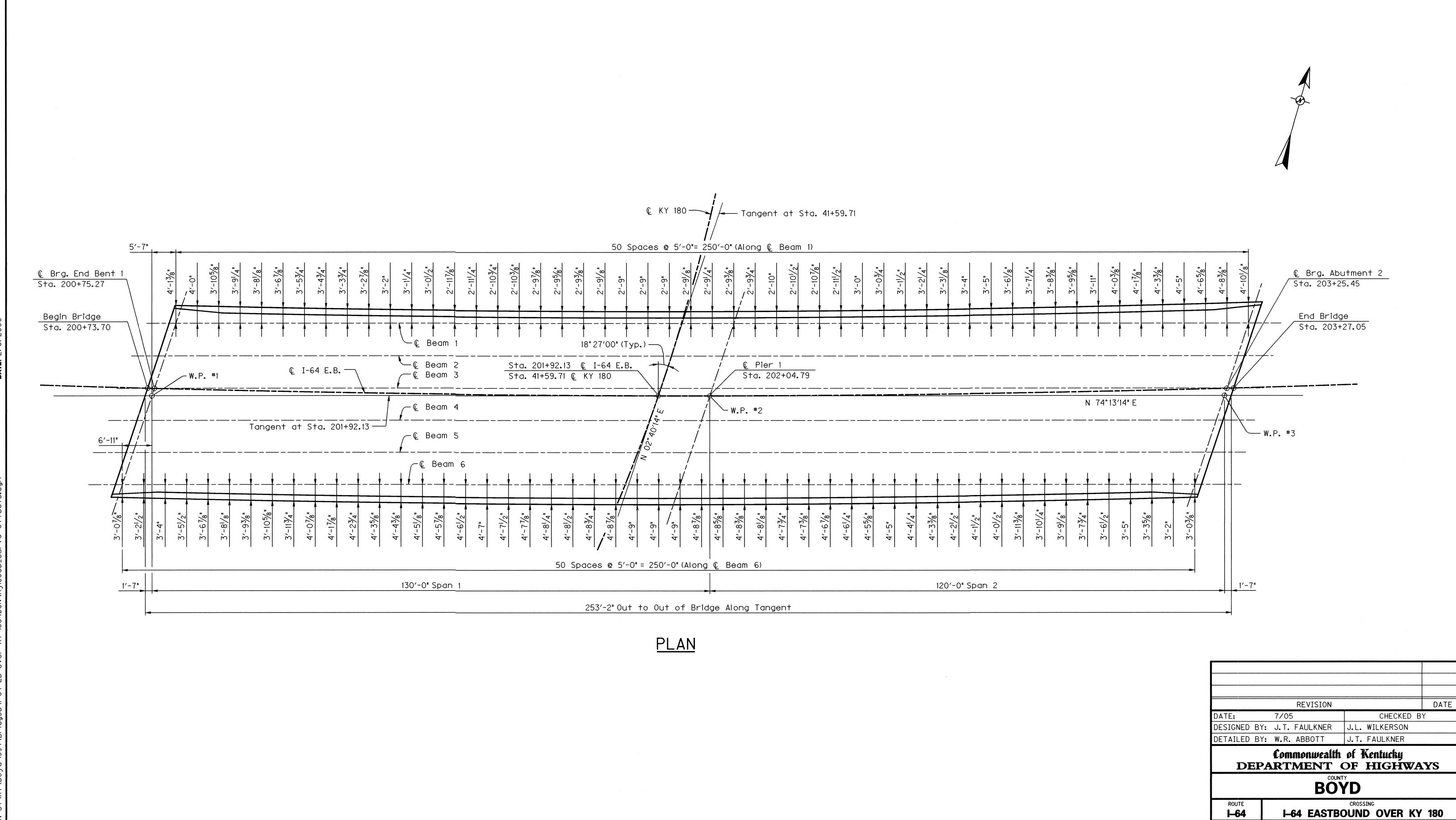












CURVE OFFSETS

LOCHNER

H.W. LOCHNER, INC.
CONSULTING ENGINEERS AND PLANNERS

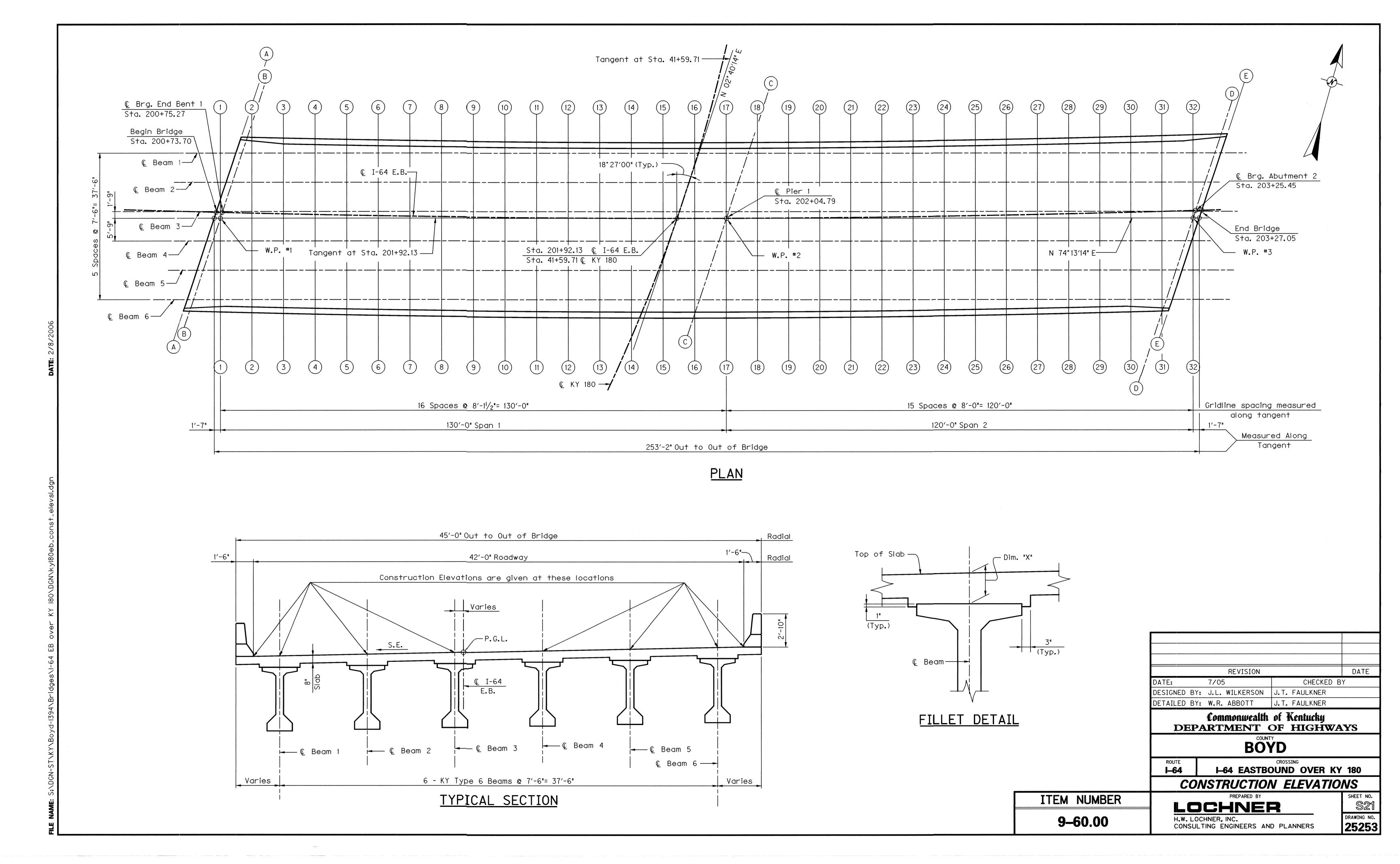
ITEM NUMBER

9-60.00

sheet no. \$20

DRAWING NO. **25253**

EILE NAME: S. UDGN-STIKY\BOXZ-1394\Briddes\|-64 FR over KY 180\DGN\kx|80eh Clirve



									CONSTE	RUCTIO	ON ELEV	ATIONS	2								
ODID			© BEAM 1		Ţ Ç	BEAM 2		(BEAM 3		Q	BEAM 4			© BEAM 5		Ţ Ç	BEAM 6	-	RIGHT	ODID
GRID LINE	LEFT GUTTER	CONST. ELEV.	TOP OF BEAM	DIM.	CONST. ELEV.	TOP OF BEAM	DIM. "X"	CONST. ELEV.	TOP OF BEAM	DIM. "X"	CONST. ELEV.	TOP OF BEAM	DIM.	CONST. ELEV.	TOP OF BEAM	DIM.	CONST. ELEV.	TOP OF BEAM	DIM. "X"	GUTTER	GRID LINE
Α	692.588	692.679			692.940				693.201		693.463			693.724	<i>DE/</i>		693.986			694.040	A
В	692.636	692.727			692.987				693.248		693.510			693.771			694.033			694.088	В
С	696.662	696.705			696.956				697.208		697.460			697.712			697.964			698.073	С
D	700.413	700.522			700.764				701.006		701.249			701.492		***************************************	701. 735			701.786	D
E	700.463	700.573			700.815				701.057		701.300			701.543			701.786			701.835	E
1											693.546			693.925			694.273			694.354	1
2	692.696	692.816			693.163				693.501		693.828			694.206			694.554			694.643	2
3	692.990	693.100			693.447				693.783		694.109			694.486			694.832			694.929	3
4	693.280	693.382			693.728				694.063		694.387			694.763			695.106			695.211	4
5	693.568	693.662			694.006				694.339		694.662			695.036			695.377			695.487	5
6	693.849	693.938			694.280				694.610		694.931			695.302			695.640			695.756	6
7	694.126	694.207			694.548				694.876		695.194			695.561			695.897			696.018	7
8	694.395	694.472			694.809				695.135		695.449			695.815			696.148			696.273	8
9	694.658	694.728			695.063				695.386		695.697			696.060			696.391			696.520	9
10	694.912	694.978			695.310				695.630		695.940			696.299			696.626			696.760	10
11	695.160	695.221			695.550				695.868		696.174			696.531			696.856			696.992	11
12	695.399	695.457			695.783				696.099		696.402			696.758			697.079			697.217	12
13	695.631	695.688			696.012				696.324		696.624			696.977			697.298			697.437	13
14	695.858	695.911			696.231				696.542		696.841			697.192			697.512			697.652	14
15	696.078	696.129			696.450				696.758		697.056			697.405			697.722			697.863	15
16	696.295	696.346			696.664				696.970		697.267			697.616			697.933			698.073	16
17		696.559			696.876				697.183		697.496			697.866			698.206			698.351	17
18	696.732	696.791			697.129				697.458		697.776			698.145			698.483			698.627	18
19	697.011	697.072			697.410				697.737		698.054			698.422			698.759			698.900	19
20	697.289	697.352			697.688				698.014		698.329			698.697			699.032			699.170	20
21		697.630			697.964				698.289		698.603			698.968			699.300			699.434	21
22		697.904			698.237				698.560		698.871			699.233			699.564		***************************************	699.692	22
23		698.173			698.505		***		698.825		699.133			699.493			699.821			699.946	23
24		698.437			698.767				699.084		699.391			699.748			700.074			700.192	24
25	 	698.695			699.022				699.337		699.641			699.997			700.320			700.431	25
26		698.948			699.272				699.585		699.887			700.240			700.561		***************************************	700.666	26
27	 	699.195			699.517				699.828		700.128			700.478			700.796			700.894	27
28		699.437			699.757				700.065		700.362			700.710			701.027			701.117	28
29		699.673			699.991				700.297		700.593			700.940			701.255			701.337	29
30		699.906			700.221				700.526		700.820			701.166			701.480			701.553	30
31		700.136			700.450				700.753		701.047			701.391			701.705			701.768	31
32	700.228	700.363			700.676	***************************************			700.979												32
									1					1							

- 1.) Take elevations on top of beam at points indicated after diaphragms are in place and after forms for concrete slabs have been in place. Read elevations to three decimals using a target rod and enter reading in table under Top of Beam Elevations.
- 2.) Compute dimension "X" as follows: Construction Elevation minus Top of Beam Elevation equals Dimension "X". Construction Elevations include camber due to weight of concrete slab, barrier, and future surfacing. Measuring of Dimension "X" gives the final check on beam tolerances for camber, beam damage and errors in erection that produce reverse camber, sags, and unsightly fascia beams.
- 3.) For setting templates, measure dimension "X" above top of beam for top of template. Do not set template by elevation.
- 4.) Construct barrier to roadway grade. Do not add camber to barrier.
- 5.) Note To Resident: The "Maximum Allowable Camber" shown on the beam sheet is the amount of camber, measured prior to casting the deck, above which the beam will begin to encroach into the slab.

ITEM NUMBER

9-60.00

REVISION			DATE
DATE:	11/05	CHECKED B	SY
DESIGNED BY:	J.L. WILKERSON	J.T. FAULKNER	
DETAILED BY:	W.R. ABBOTT	J.T. FAULKNER	

Commonwealth of Kentucky DEPARTMENT OF HIGHWAYS

BOYD

I-64 EASTBOUND OVER KY 180

CONSTRUCTION ELEVATIONS

LOCHNER

H.W. LOCHNER, INC. CONSULTING ENGINEERS AND PLANNERS

sheet no. \$22 DRAWING NO. 25253

